CONSTRUCTION SPECIFICATIONS & CONTRACT DOCUMENTS

EVANS COUNTY FIREHOUSE SLAB

Prepared for:

EVANS COUNTY COMMISSIONERS

PROJECT NO: PE14134

AUGUST 2023

PARKER Engineering

36 Courtland Street, Suite B
Statesboro, GA 30458

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END OF SECTION

SECTION 00100
ADVERTISEMENT FOR BIDS

PART 1. GENERAL

1.1 SEALED BIDS

Sealed bids for construction of the **Evans County Firehouse Slab Project** will be received until **August 31st, 2023**, **2:00 PM** at the **Evans County Commissioners Office** at **613 West Main Street, Claxton, GA 30417** at which time and place they will be publicly opened and read. **Any bid received after said time and date will not be considered by the Owner.** No bid may be withdrawn after the closing time for the receipt of bids for a period of **sixty (60)** days.

1.2 SCOPE OF WORK

The work shall consist of furnishing all products and equipment and performing all labor necessary to perform the following:

Constructing 11,080 SY concrete slab and footers at the Evans County Firehouse. The County will be providing the concrete.

Time allotted for completion of work is **thirty (30)** calendar days. Evans County will require the Contractor to start the work by September 1, 2023.

1.3 PLANS, SPECIFICATIONS AND CONTRACT DOCUMENTS

Plans, Specifications and Contract Documents are on file at Parker Engineering, LLC. Copies may be obtained from **Parker Engineering, LLC.**, 36 Courtland Street, Suite B, Statesboro, Georgia 30458, Phone: (912) 764-7722, Email: lindsey@parker-engineering.com, upon payment of $50.00 for each full-size set (non-refundable).

1.4 FUNDING SOURCES

Funding is to be provided by the Evans County Commissioners.

1.5 RESERVATION OF RIGHTS

Owner reserves the right to reject any or all Bids and to waive informalities, including without limitation, the rights to reject any or all nonconforming, non-responsive, unbalanced or conditional Bids and to reject the Bid of any Bidder if Owner believes that it would not be in the best interest of the Project to make an award to that Bidder, whether because the Bid is not responsive or the Bidder is unqualified or of doubtful financial ability or fails to meet any other pertinent standard or criteria established by the Owner.

1.6 E-VERIFY

Contractor shall comply with E-Verify regulations.

EVANS COUNTY COMMISSIONERS
OWNER

END OF SECTION

SECTION 00500
AGREEMENT

THIS AGREEMENT is dated as of the ___ day of _____ in the year 20___, by and between the **EVANS COUNTY COMMISSIONERS** (hereinafter called OWNER) and. (hereinafter called CONTRACTOR).

OWNER and CONTRACTOR, in consideration of the mutual covenants hereinafter set forth, agree as follows:

ARTICLE 1. WORK

1.1 CONTRACTOR shall complete all Work as specified or indicated in the Contract Documents. The Work is generally described as follows:

Constructing 11,080 SY concrete slab and footers at the Evans County Firehouse. Evans County will be provide the concrete.

1.2 The Project for which the Work under the Contract Documents may be the whole or only a part is generally described as follows:

EVANS COUNTY FIREHOUSE SLAB

ARTICLE 2. ENGINEER

2.1 The Project has been designed by Parker Engineering, LLC. who is hereinafter called ENGINEER and who is to act as OWNER's representative, assume all duties and responsibilities and have the rights and authority assigned to ENGINEER in the Contract Documents in connection with completion of the Work in accordance with the Contract Documents.

ARTICLE 3. CONTRACT TIME

3.1. CONTRACTOR agrees to commence Work under this Agreement on or before a date to be specified on a written "Notice to Proceed" of the OWNER and to substantially complete the work in **25 consecutive calendar days** and to fully complete the Work within **30 consecutive calendar days** from the "Notice to Proceed" date.

ARTICLE 4. CONTRACT PRICE.

Lump Sum Work

OWNER shall pay CONTRACTOR for completion of the Work in accordance with the Contract Documents an amount in current funds of the amounts determined for all lump sum work, said amount being:

_____. ($_____)

ARTICLE 5. PAYMENT PROCEDURES

Applications for Payment will be processed by the ENGINEER.

5.1 Progress Payments; Retainage. OWNER shall make progress payments on account of the Contract Price on the basis of CONTRACTOR'S Applications for Payment as recommended by ENGINEER, 30 days after the receipt of an invoice for the performance of the Work. All such payments will be measured by the schedule of values as provided by the CONTRACTOR and approved by the ENGINEER. Retainage will be held at a rate of 5% of the work completed.

5.2 Final Payment. Upon final completion and acceptance of the Work the OWNER shall pay the remainder of the Contract Price as recommended by the ENGINEER.

ARTICLE 6. INTEREST

6.1 There will be no interest earned or paid on retainage.

ARTICLE 7. CONTRACTOR'S REPRESENTATIONS.

In order in induce OWNER to enter into this Agreement CONTRACTOR makes the following representations:

7.1 CONTRACTOR has familiarized itself with the nature and extent of the Contract Documents, Work, site, locality, and all local conditions and Laws and Regulations that in any manner may affect cost, progress, performance or furnishing of the Work.

7.2 CONTRACTOR has given ENGINEER written notice of all conflicts, errors or discrepancies that he has discovered in the Contract Documents and the written resolution thereof by ENGINEER is acceptable to CONTRACTOR.

ARTICLE 8. CONTRACT DOCUMENTS

8.1 This Agreement

8.2 Exhibits to this Agreement: None

8.3 Notice of Award

8.4 Drawings

8.5 Specifications bearing the title **EVANS COUNTY FIREHOUSE SLAB** consisting of 9 divisions, as listed in table of contents thereof.

8.6 Addenda numbers ___ to ___, inclusive.

8.7 CONTRACTOR's Bid

ARTICLE 9. NOT USED.

ARTICLE 10. MISCELLANEOUS

10.1. No assignment by a party hereto of any rights under or interests in the Contract Documents will be binding on another party hereto without the written consent of the party sought to be bound; and specifically but without limitation moneys that may become due and moneys that are due may not be assigned without such consent (except to the extent that the effect of this restriction may be limited by law), and unless specifically stated to the contrary in any written consent to an assignment no assignment will release or discharge the assignor from any duty or responsibility under the Contract Documents.

10.3 OWNER and CONTRACTOR each binds itself, its partners, successors, assigns and legal representatives to the other party hereto, its partners, successors, assigns and legal representatives in respect of all covenants, agreements and obligations contained in the Contract Documents.

10.4 Complete Agreement. This Agreement, including the Contract Documents, contains all of the understandings and agreements of whatsoever kind and nature existing between the parties hereto with respect to the subject matter contained herein.

10.5 Governing Law. This Agreement shall be governed by and construed under the laws of the State of Georgia.

10.6 Counterparts. This Agreement may be executed in any number of counterparts, each of which shall be deemed to be an original, but all of which together shall constitute one and the same instrument.

10.7 Any provision or part of the Contract documents held to be invalid or unenforceable under any Law or Regulation shall be deemed stricken, and all remaining provisions shall continue to be valid and binding upon OWNER and CONTRACTOR, who agree that the Contract documents shall be reformed to replace such stricken provision or part thereof with a valid

and enforceable provision that comes as close as possible to expressing the intention of the stricken provision.

10.8 Notice. All notices, requests, demands and other communications hereunder shall be in writing and shall be deemed received, and shall be effective when personally delivered or on the third day after the postmark date when mailed by certified mail, postage prepaid, return receipt requested or upon actual delivery when sent *via* national overnight commercial carrier to the parties at the addresses given below, unless a substitute address shall first be furnished to the other parties by written notice in accordance herewith:

NOTICE TO OWNER shall be sent to:

Evans County Commissioners
613 West Main Street
Claxton, GA 30417

NOTICE TO CONTRACTOR shall be sent to:

_____
_____
_____

10.9 Force Majeure. Neither the Owner nor the Contractor shall be liable for their respective non-negligent or non-willful failure to perform or shall be deemed in default with respect to the failure to perform (or cure a failure to perform) any of their respective duties or obligations under this Agreement or for any delay in such performance due to: (i) any cause beyond their respective reasonable control; (ii) any act of God; (iii) any change in applicable governmental rules or regulations rendering the performance of any portion of this Agreement legally impossible; (iv) earthquake, fire, explosion or flood; (v) strike or labor dispute, excluding strikes or labor disputes by employees and/or agents of Engineer; (vi) delay or failure to act by any governmental or military authority; or (vii) any war, hostility, embargo, sabotage, civil disturbance, riot, insurrection or invasion. In such event, the time for performance shall be extended by an amount of time equal to the period of delay caused by such acts and all other obligations shall remain intact.

10.10 Headings. All headings herein are inserted only for convenience and ease of reference and are not to be considered in the construction or interpretation of any provision of this Agreement.

ARTICLE 11. OTHER PROVISIONS

IN WITNESS WHEREOF. OWNER and CONTRACTOR have signed this Agreement in three (3) counterparts. One counterpart each has been delivered to OWNER, CONTRACTOR, and ENGINEER. All portions of the Contract Documents have been signed or identified by OWNER and CONTRACTOR or by ENGINEER on their behalf.

EVANS COUNTY COMMISSIONERS

_____
Signature
Casey Burkhalter, County Administrator

Attest _____

CORPORATE SEAL

_____
Contractor

_____
By

_____
Signature

_____
Title

Attest _____

CORPORATE SEAL

END OF SECTION

SECTION 00618
CERTIFICATE OF INSURANCE

1. Contractor shall supply proof of commercial liability insurance and workman's compensation insurance.

SECTION 00621
NOTICE OF AWARD

PROJECT DESCRIPTION:

EVANS COUNTY FIREHOUSE SLAB

The OWNER has considered the BID submitted by you on _____ for the above described WORK in response to its Advertisement for Bids and Instruction to Bidders.

You are hereby notified that your BID has been accepted for items in the amount of _____($_____).

You are required by the Instructions to Bidders to execute the Agreement and furnish the required CONTRACTOR's Performance BOND, Payment BOND and Certificates of Insurance within fifteen (15) calendar days from the date of the Notice to you.

If you fail to execute said Agreement and to furnish said BONDS within fifteen (15) days from the date of this notice, said OWNER will be entitled to consider all your rights arising out of the OWNER'S acceptance of your BID as abandoned and as a forfeiture of your BID BOND. The OWNER will be entitled to such other rights as may be granted by law.

You are required to return an acknowledged copy of this NOTICE of AWARD to the OWNER.

Dated this _____ day of _____, 20___.

EVANS COUNTY COMMISSIONERS

_____
Signature

By: Casey Burkhalter
Title: County Administrator

ACCEPTANCE OF NOTICE

Receipt of the above NOTICE OF AWARD is hereby acknowledged this ____ day of _____, 20__.

_____
Signature

By: _____

Title: _____

SECTION 00622
NOTICE TO PROCEED

To: _____.___ Date: _____

Project: Evans County Firehouse Slab

You are hereby notified to commence WORK in accordance with the Agreement dated _____on or before _____ and you are to substantially complete the WORK within __25__ consecutive calendar days thereafter. The date of substantial completion of all WORK is therefore_____. You are required to complete all work within ___30__ consecutive calendar days from _____ . The date of completion for all work is therefore _____ .

EVANS COUNTY COMMISSIONERS

By: Casey Burkhalter

_____
Signature

Title: County Administrator

ACCEPTANCE OF NOTICE

Receipt of the above NOTICE
TO PROCEED is hereby
acknowledged by:

Signature:_____

Date: _____

Title: _____

SECTION 00840
ADMINISTRATIVE AND PROCEDURAL ITEMS AND FORMS

1. Certificate of Substantial Completion
2. Release by Claimants
3. Contractor's Guarantee

Certificate of Substantial Completion

Project:	Owner:	Owner's Contract No.:
Evans County Firehouse Slab	Evans County Commissioners	N/A
Contract:		Date of Contract:
Contractor:		Engineer's Project No.: PE14134

This [tentative] [definitive] Certificate of Substantial Completion applies to:

☐ All Work under the Contract Documents: ☐ The following specified portions:

Date of Substantial Completion

The Work to which this Certificate applies has been inspected by authorized representatives of Owner, Contractor and Engineer, and found to be substantially complete. The Date of Substantial Completion of the Project or portion thereof designated above is hereby declared and is also the date of commencement of applicable warranties required by the Contract Documents, except as stated below.A [tentative] [revised tentative] [definitive] list of items to be completed or corrected, is attached hereto. This list may not be all-inclusive, and the failure to include any items on such list does not alter the responsibility of the Contractor to complete all Work in accordance with the Contract Documents.

The responsibilities between OWNER and CONTRACTOR for security, operation, safety, maintenance, heat, utilities, insurance, and warranties shall be as provided in the Contract Documents except as amended as follows:

☐ Amended Responsibilities ☐ Not Amended

Owner's Amended Responsibilities:

Contractor's Amended Responsibilities:

The following documents are attached to and made part of this Certificate:

This Certificate does not constitute an acceptance of Work not in accordance with the Contract Documents nor is it a release of Contractor's obligation to complete the Work in accordance with the Contract Documents.

Executed by Engineer — Date

Accepted by Contractor — Date

Accepted by Owner — Date

RELEASE OF LIENS FROM CONTRACTOR

To Whom it May Concern,

This is to certify that we, _____, have been partially paid for labor and materials furnished to the **Evans County Commissioners** for the **Evans County Firehouse Slab Project**. Effective upon receipt of the final payment of $_____, the undersigned forever waives, releases and relinquishes any and all claims or rights of liens in any way connected with the above named job.

Signature: _____

Printed Name: _____

Title:_____

Sworn and subscribed to me this _____ day of _____, 20__.

_____
Notary Public

Commission Expires:_____

CONTRACTOR'S GUARANTEE

GUARANTEE FOR **Evans County Commissioners**
Evans County Firehouse Slab
Project No.: PE14134

We hereby guarantee that the **Evans County Firehouse Slab Project** which we have installed as work done under the above contract has been done in accordance with the drawings and specifications and that the work as installed will fulfill the requirements of the guarantee included in the specifications. We agree to repair or replace any or all of our work together with any other adjacent work which may be displaced in so doing, that may prove to be defective in its workmanship or material within a period of one year from the date of acceptance (_____) by the Owner, of the work included under the above contract, without any expenses whatsoever to the Owner.

Within 10 days after being notified in writing by the Owner of any defects in the work we agree to commence and prosecute with due diligence all work necessary to fulfill terms of the guarantee, and to complete the work within a reasonable period of time, in the event of our failure to so comply, Owner will proceed to have such work done at our expense and we will honor and pay the cost and charges thereof upon demand.

We agree that, in case of emergency, where, in the opinion of the Owner, delay would cause serious loss or damage to the Owner, repairs may be made without notice, and we collectively and separately, agree to pay the cost thereof upon demand.

DATED: _____

PRIME CONTRACTOR SIGNATURE:_____

SUBCONTRACTOR SIGNATURE:_____

State of Georgia
County of _____

Personally appeared before this _____ day of _____, _____ the undersigned and who after being duly sworn, depose, and say that the acts stated in the above affidavit are true.

Notary Public:_____

Commission Expiration Date:_____
(NOTARY SEAL)

SECTION 02040
SCOPE OF WORK

EVANS COUNTY FIRE STATION CONCRETE SLAB AND FOOTERS

PART 1. GENERAL INFORMATION

1.1 OWNER

Evans County Commissioners
3 Freeman Street,
Claxton, GA 30417

1.2 PURPOSE

A. To construct 11,080 square foot slab and footers of a 3-bay fire station with living quarters.

1.3 LOCATION

A. The intersection of Deloach Street and James Street in Claxton, GA.

PART 2. SCOPE OF WORK

1.1 MATERIALS

A. Concrete will be provided by Evans County. (Contractor to provide coordination.)

1.2 SCOPE OF WORK PROVIDED BY EVANS COUNTY

A. Provide four building corner locations in the field.
B. Construct the subgrade within 0.2-feet of finished subgrade elevation.
C. Provide testing services (Contractor to coordinate with testing service.)
D. Termite treatment. (Contractor to coordinate with provider.)
E. Plumbing and electrical.

1.3 SCOPE OF WORK PROVIDED BY CONTRACTOR

A. Provide layout and surveying services required to construct the building. County will supply four builder corners.
B. Demobilize to allow plumbing and electrical contractors to perform their work.
C. Construct footers and turndowns including forming and rebar.
D. Fine grade the subgrade.
E. Add a 6-mil poly sheet. Sheet should be taped where poly laps on itself.

F. Pour foundation footers and slab. The proposed slab is a "monolithic pour turndown." Evans County will purchase the concrete. The Contractor will coordinate with the concrete company.
G. Add base plates.
H. Provide contraction joints at a 12-feet x 12-feet spacing.
I. Facilitate testing.
J. Provide all labor, equipment, and materials (excluding concrete) required to perform the job including light towers, concrete finishing equipment, etc.

PART 3. ESTIMATED QUANTITIES

1.1 DESCRIPTION

A. The quantities below are provided as a courtesy to the Bidder to demonstrate how large the project is. The Bidder shall use the attached drawings to compute their own quantities.

i. Total Slab Area: 11,080 square feet
ii. 4" Slab Area: 4800 square feet
iii. 8" Slab Area: 6280 square feet
iv. Square Footers with Piers: 4
v. Square Footers: 18
vi. Base Plates with Anchor Bolts: 34

SECTION 02522
CONCRETE PAVING, WALKS AND CURBS

PART 1. GENERAL

1.1 SCOPE

A. The work included in this section includes furnishing all materials, equipment, and labor necessary to construct Portland Cement Concrete paving, sidewalks, driveways, and curb and gutter, as shown on the construction plans.

1.2 REFERENCES

A. ACI031 – Specifications for Concrete Buildings.

B. ACI304 – Recommended Practice for Measuring, Measuring, Mixing, Transporting, and Placing Concrete.

C. ASTM A 185-94 – Steel Welded Wire Fabric, Plain, for Concrete Reinforcement.

D. ASTM A 497-94 – Steel Welded Wire Fabric, Deformed, for Concrete Reinforcement.

E. ASTM A 615-95 – Deformed and Plain Billet-Street Bars for Concrete Reinforcement.

F. ASTM C 33-93 – Concrete Aggregates.

G. ASTM C 39-94 – Compressive Strength of Cylindrical Concrete Specimens

H. ASTM C 94-94 – Ready-Mixed Concrete.

I. ASTM C 150-05 – Portland Cement

J. ASTM C 260-94 – Air – Entraining Admixtures for Concrete.

K. ASTM C 309-94 – Liquid Membrane – Forming Compounds for Curing Concrete.

L. ASTM C 494-92 – Chemical Admixtures for Concrete

M. ASTM D 1751-83 (1991) – Preformed Expansion Joint Filler for Concrete Paving and Structures Construction. (Nonextruding and Resilient Bituminous

Type).

N. ASTM D 1752-84 (1992) – Performed Sponge Rubber and Cork Expansion Joint Fillers for Concrete Paving and Structural Construction.

O. ASTM D 3740-94 – Minimum Requirements for Agencies Engaged in the Testing and/or Inspection of Soil and Rock as Used in Engineering Design and Construction.

P. ASTM E 329-93 – Agencies Engaged in the Testing and/or Inspection of Materials used in construction.

1.3 REGULATORY REQUIREMENTS

A. Perform work in accordance with State of Georgia, and the Department of Transportation, State of Georgia, Standard Specifications, Current Edition.

B. Conform to applicable standards for paving work on public property.

1.4 TESTING

A. All sampling and testing services shall be performed by a testing agency that operates in accordance to ASTM D 3740 and E 329 latest revision. Contractor shall hire the testing agency.

B. The testing laboratory and Project Engineer/Project Representative shall be given a minimum of 48 hours notice prior to taking any of the tests.

C. Testing shall be the responsibility of the Contractor and shall be performed at the Contractor's expense by a commercial testing laboratory.

D. The Contractor shall submit to the Engineer, for review, the concrete materials and the concrete mix designs for each class of concrete proposed for use. This submittal shall include the results of all testing performed to qualify the materials and establish the mix designs. All mix designs shall be proportioned in accordance with Section 3.9 of ACI 301, Method 1 (trial batches) or method 2 (field experience). The average strength used as the basis for selecting proportions shall be specified in paragraph 3.9.2 or ACI 301.

E. The testing laboratory shall conduct strength tests of the concrete during construction in accordance with Section 16.3.4 of ACI 301. At least one strength test (6 test cylinders) shall be made for reach 50 cubic yards or fraction thereof, of each mix design of concrete placed in any 1 day.

F. Slump tests shall be conducted as concrete is discharged from mixer at the point of placing in accordance with Section 16.3.5 of ACI 301. Slump tests will be made of every batch of concrete placed, and failure to meet specified slump requirements will be sufficient cause for rejection of that batch.

G. The air content of the concrete sample for each strength test shall be determined in accordance with Section 16.3.6 of ACI 301.

H. Results of all tests shall be submitted to the Engineer, with copies to the Contractor. The test reports shall include the exact location in the work at which the batch represented by a test was deposited.

I. Evaluation of test results and acceptance of concrete shall be in accordance with chapter 17 of ACI 301.

1.5 SUBMITTALS - None

1.6 WEATHER CONDITIONS

A. Do not place concrete when base surface temperature is less than 40° F, or surface is wet or frozen.

PART 2. PRODUCTS

2.1 FORM MATERIALS

A. Shall conform to ACI301.

B. Shall be wood, plywood, metal or other accepted material and shall be of the grade or type suitable to obtain the finish specified. The material should be of size and strength to resist movement during concrete placement and to retain horizontal and vertical alignment until removed. Use straight forms, free of distortion and defects.

C. Use flexible spring steel forms or laminated boards to form radius bends as required.

D. Form Ties: Removable or snap-off type, galvanized metal, adjustable length, free of defects that could leave holes larger than 1-inch in concrete surface.

E. Form Release Agent: Colorless mineral oil, which will not stain concrete, or absorb moisture, or impair natural bonding or color characteristics of coating intended for use on concrete.

2.2 REINFORCING STEEL

A. Reinforcing Steel: ASTM A 615, Grade 60 billet steel deformed bars; uncoated finish.

B. Welded Steel Wire Fabric: Plain type, ASTM A 185; uncoated finish.

C. When specifically called for on the plans, reinforcing bars shall be epoxy-coated in accordance with ASTM A775.

D. Bar Supports and Accessories: Bar supports and accessories shall be galvanized or plastic-coated wire conforming with the requirement of ACI 315, Chapter 7, and/or CRSI MSP, and shall be specifically made for the intended use by proprietary manufacturers.

2.3 CONCRETE MATERIALS

A. Cement: Portland, Type I per ASTM C 150.

B. Fine and Coarse Mix Aggregates: ASSTM C33.

C. Water: Potable, not detrimental to concrete

D. Air Entrainment: ASTM C260.

2.4 JOINT SEALERS

A. Joint Fillers: ASTM D1751 Asphalt impregnated fiberboard or felt, tongue and grove profile.

B. Sealant: Joints shall be sealed per detail on project drawings. ASTM C 920-94, Type S or M, Grade P or NS, Class 25.

2.5 CONCRETE MIX-BY PERFORMANCE CRITERIA

A. Concrete mixed at the job site shall be mixed in a batch mixer in accordance with American Concrete Institution Standard A.C.I 318, and in a similar manner subject to acceptance. Ready-mixed concrete shall conform to A.S.T.M. Standard C-94. Mixing time for stationary mixers over 1 cubic yard in capacity shall be increased 15 seconds for each additional ½ cubic yard or fraction thereof materials mixed. Construction shall be in accordance with applicable portions of "Building Code Requirements for Concrete" (A.C.I. – 318).

B. Provide concrete to the following criteria in accordance with ACI 318, Section 5.3:

1. Maximum water-cement ratio:
   0.45 (lbs/lb) – Class A Concrete
   0.55 (lbs/lb) – Class B Concrete

2. Air Content: 5% $\pm$ 1.5% (Class A & B)

3. Fiber: 1.5 lb per cubic yard

4. Slump: 4" $\pm$ 1: (Class A & B)

5. Minimum compressive strength at 28 days:
   4,000 psi – Class A: Structural, slabs on grade, paving
   3,000 psi – Class B: Curbs and sidewalks

C. Use accelerating admixtures in cold weather only when approved by Engineer. Use of admixtures will not relax cold weather placement requirements.

D. Use set retarding admixtures during hot weather only when approved by Engineer.

2.6 PAVEMENT MARKINGS

A. Cleaning: Sweep and clean surface to eliminate material and dust.

B. Pavement Markings for Parking Lots: Use traffic lane-marking paint, factory-mixed, quick-drying, and non-bleeding (Must meet GDOT 870). Striping lanes shall be as detailed on the plans.

1. Do not apply lane marking paint until layout and placement has been verified.

2. Apply paint with mechanical equipment to produce uniform straight edges. Apply in 2 coats at manufacturer's recommended rates.

3. Parking lines shall be white.

PART 3. EXECUTION

3.1 CONSTRUCTION OBSERVATION

A. Verify compacted subgrade & granular base are acceptable and ready to support paving and imposed loads.

B. Verify gradients and elevations of base are correct.

C. The Engineer will have the right to require that any portion of the work be done in his presence and if the work is covered up after such instruction, it shall be exposed by the Contractor for observation. However, if the Contractor notifies the Engineer that such work is scheduled, and the Engineer fails to appear within 48 hours, the Contractor may proceed without him. All work done and materials furnished shall be subject to review by the Engineer or GSU project representative. Improper work shall be reconstructed. All materials, which do not conform to the requirements of the specifications, shall be removed from the work upon notice being received from the Engineer for the rejection of such materials. The Engineer shall have the right to mark rejected materials so as to distinguish them as such.

3.2 SUBBASE

A. Prepare subbase in accordance with Section 300 - Standard Specifications for Base and Subbase Courses, Department of Transportation, State of Georgia, current Edition.

B. Remove loose material from compacted subbase surface immediately before placing concrete.

C. Proof-roll prepared subbase surface to check for unstable areas and need for additional compaction. Do not begin paving work until such conditions have been corrected and area is ready to receive paving.

3.3 PREPARATION FOR PLACEMENT

A. Water shall be removed from excavations before concrete is deposited. Hardened concrete debris, and other foreign materials shall be removed from the interior of forms and from the inside of mixing and conveying equipment. The reinforcement shall be made secure in position and shall be subject to examination and acceptance.

B. Moisten base to minimize absorption of water from fresh concrete.

C. Notify Engineer minimum 48 hours prior to commencement of concreting operations.

3.4 FORMING

A. Place and secure forms to correct location, dimension, profile, and gradient.

B. Assemble formwork to permit easy stripping and dismantling without damaging concrete.

C. Place joint filler in position, in straight lines. Secure to formwork during concrete placement.

D. Forms shall be constructed to the shape, form line, and grade required and shall be maintained sufficiently rigid to prevent deformation under load. Formwork and details of construction shall conform to ACI-318, Chapter 6.

E. Check completed formwork for grade and alignment to following tolerances:

Top of forms not more than 1/8" in 10'.

Vertical face on longitudinal axis, not more than ¼" in 10'.

F. City or City designated representative shall inspect formwork with respect to slope and thickness before all pours.

3.5 REINFORCING

A. Reinforcement shall be placed in accordance with the Plans.

B. Only reinforcement that is free of oil, dirt, loose mortar, mud or other non-metallic coatings which reduce bonding capacity shall be installed. After placing, the reinforcement shall be maintained in a clean condition until the concrete is placed.

C. All intersections of the reinforcement shall be securely tied with 16-gauge minimum, black annealed wire. Crossing bars shall not be tack welded.

D. Reinforcement supports shall be as specified and shall be supported on non-corrodible metal or plastic-encased spacers, bolsters or chairs. For concrete placement on grade, reinforcement may be supported on precast concrete blocks spaced to maintain required cover, but only where the Contractor can demonstrate that the precast blocks are at least equal in quality to the class concrete specified for the work.

E. Bars that are partially embedded in concrete shall not be field bent unless concurrence has been obtained from the Engineer. Procedure used shall be based on the Contractor's report and recommendations.

F. Unless shown otherwise on the Plans, the following minimum concrete cover shall be provided for reinforcement:

		Minimum Cover, Inches
1.	Concrete exposed to weather	3″
2.	Concrete cast against earth	3″
3.	#6 and larger bars	2″
4.	All #5 and smaller bars	1-1/2″

The maximum cover shall be no greater than that specified above or shown plus 3/8 inch.

3.6 PLACING CONCRETE

A. Placing of concrete shall conform to Chapter 5 of the American Concrete Institution Standard A.C.I. 318. Concrete having attained initial set or having contained water for more than 45 minutes shall not be used in the work. Concrete shall not be dropped freely more than 3 feet. Concrete shall be mixed and placed only when the temperature is at least 40 degrees F and rising. Concrete shall be placed only upon surfaces that are free from frost, ice, mud and other detrimental circumstances. When placed on dry soil or previous materials, waterproof paper or polyethylene sheeting shall be laid over the surfaces that are to receive the concrete.

B. Ensure reinforcement, inserts, embedded parts, and formed joints are not disturbed during concrete placement.

C. Place concrete continuously over the full width of the panel and between predetermined construction joints. Do not break or interrupt successive pours such that cold joints occur.

D. Use bonding agent at locations where fresh concrete is placed against hardened or partially hardened concrete surfaces.

E. Curbs and Gutters: Automatic machine may be used for curb and gutter placement at Contractor's option. If machine placement is to be used, submit revised mix design and laboratory test results which meet or exceed minimum specified. Machine placement must produce curbs and gutters to required cross-section, lines, grades, finish and jointing as specified for formed concrete. If results are not acceptable, remove and replace with formed concrete as specified.

F. When curbing is installed on an existing road, the Contractor shall saw cut

the asphalt to produce a straight curb / asphalt interface.

G. When curbing is installed on an existing but not directly adjacent to the existing asphalt, the Contractor shall remove enough asphalt to allow the width of a paving machine to complete the job. As an alternative, the Contractor may saw cut a straight line in the existing asphalt and then build the curb, gutter and road with one pour of concrete.

3.7 JOINTS

A. The Contractor shall submit a detailed joint layout and placing sequence for approval of the engineer prior to proceeding.

B. Construct joints true-to-line with face perpendicular to concrete surface. Construct transverse joints at right angles to the centerline, unless otherwise indicated. When joining existing structures, place transverse joints to align with previously placed joints, unless otherwise indicated.

C. Contraction Joint: Provide contraction joints at 12 ft. O.C. maximum. Joints shall be saw-cut, ¼" wide and have a depth of ¼ of slab thickness, or 1" minimum. Joints shall be made within 4 to 12 hours after slab has been finished, depending on temperature.

1. Transverse contraction joints for sidewalks and curbs shall be placed at 10 ft. intervals (maximum) and have rounded edges with ¼" radius. Joints shall be hand-formed or saw-cut.

2. Caulking of saw joints in the concrete parking lot is **NOT** required.

D. Expansion Joint: Provide 3/8" wide expansion joints where sidewalks meet other structures, curbs, catch basins, manholes, or other sidewalks or at 48 ft. maximum spacing. All expansion joints should be filled with an asphalt impregnated fiber board or felt and caulked as discussed in paragraph 2.4. The contractor shall submit a detailed joint layout and placing sequence for review and approval along with other shop drawing submittals.

E. Construction Joint: Provide construction joints at end of placements and at locations where placement operations are interrupted for a period of more than ½ hour.

3.8 FINISHING

A. Screeding and Floating: After striking-off and consolidating concrete, smooth surface by screeding and floating. Use hand methods only where

mechanical floating is not possible. Adjust floating to compact surface and produce uniform texture.

B. Light Broomed Finish: This shall be used for sidewalks. After floating, power trowel slabs to receive a light broomed finish to produce a smooth surface, relatively free of defects. Before the surface sets, pass a soft broom drag over the surface to produce a surface uniform in texture and appearance.

C. Troweled Finish: This shall be used for concrete drives and curbs and gutters. After floating, power trowel slabs to receive a troweled finish to produce a smooth surface, relatively free of defects. Hand trowel after the surface has hardened sufficiently. When a ringing sound is produced as the trowel is moved over the surfaces, perform final troweling by hand to produce a surface which is thoroughly consolidated, free from trowel marks, uniform in texture and appearance and plane to a tolerance of 1/8 inch to 10 feet as determined by a 10-foot straightedge placed anywhere on the slab in any direction.

D. Work edges of gutters, back top edge of curb, and formed joints with an edging tool, and round to ½" radius, unless otherwise indicated. Eliminate tool marks on concrete surface.

E. Do not remove forms for 24 hours after concrete has been placed. After form removal, clean ends of joints and point-up any minor honeycombed areas. Remove and replace areas or sections with major defects, as directed by Engineers.

3.9 CONCRETE CURING

A. Immediately after placement and finishing, concrete shall be protected from moisture loss for not less than 7 days. For concrete surfaces not in contact with forms, clear curing compound conforming to Federal Specification TT-C-800A shall be uniformly applied after water sheen disappears from the concrete surface. Formed surfaces shall receive an application of curing compound if forms are removed during the 7-day curing period. Curing compound shall not be applied during rainfall.

B. Curing compound shall be applied under pressure at the rate of 1 gallon to not more than 150 square feet by mechanical sprayers. The spraying equipment shall be of the fully atomizing type. At the time of use, the compound shall be thoroughly mixed. Care shall be taken to prevent application to joints where concrete bond is required for reinforcing steel and to joints where joint sealer is to be placed. The compound shall form a uniform continuous coherent film that will not crack or peel and shall be free

from pinholes and other imperfections. Concrete surfaces that are subjected to heavy rainfall within 3 hours after curing compound has been applied shall be resprayed by the above method and at the above coverage at no additional expense to Owner.

C. No pedestrian or vehicular traffic shall be allowed over the surface for seven days unless the surface is protected by planks, plywood, or sand. The protection shall not be placed until at least 12 hours after the application of the curing compound.

D. Protect concrete by suitable methods to prevent damage by mechanical injury or excessively hot or cold temperatures.

3.10 FIELD QUALITY CONTROL

A. Contractor will be responsible for all testing.

B. Sets of six field control cylinder specimens shall be taken for every fifty (50) cubic yards of concrete placed. During cold weather concreting, one additional test cylinder shall be taken and cured on the job site under the same conditions as the concrete it represents. Not less than one set of specimens shall be taken on any one day when concrete is being placed. One slump test shall be performed for each set of test cylinders taken and for each concrete mixer truck delivered. All specimens shall be taken in conformance with ASTM C31. When average ultimate 28-day strength of control cylinders in any set falls below the required ultimate strength or below proportional minimum 7-day strengths where proper relation between 7 and 28-day strengths have been established by tests, proportions, water content, or temperature conditions shall be changed to secure the required strength.

C. The Contractor shall cooperate in the making of such tests to the extent of allowing free access to the work for the selection of samples, providing heated (when required) moist storage facilities for specimens, affording protection to the specimens against injury or loss through his operations, and furnishing material and labor required for the purpose of taking concrete cylinder samples, curing boxes, and shipping boxes.

D. Air entrainment shall be measured by the testing laboratory at time of concrete deposit in accordance with ASTM C231.

E. Maintain records of placed concrete items. Record date, location of pour, quantity, air temperature, and test samples taken.

3.11 PROTECTION

A. Immediately after placement, protect pavement from premature drying, excessive hot or cold temperatures, and mechanical injury.

B. Repair or Replace broken or defective concrete, as directed by Engineer.

C. Allow testing company to drill test cores where directed by Engineer, when necessary to determine magnitude of cracks or defective areas. Fill drilled core holes in satisfactory pavement areas with portland cement concrete bonded to pavement with epoxy adhesive.

D. Protect concrete from damage until acceptance of work. Exclude traffic from pavement for at least 14 days after placement. When construction traffic is permitted, maintain pavement as clean as possible by removing surface stains and spillage of materials as they occur.

E. Sweep concrete curb and gutter and wash free of stains, discolorations, dirt, and other foreign material just prior to final inspection.

END OF SECTION

Approved

Casey Burkhalter

Casey Burkhalter

7-17-2023

BIGBEE STEEL BUILDINGS, INC.

P.O. BOX 2314
MUSCLE SHOALS, ALABAMA 35662

PHONE: (256) 383-7322 or (800) 633-3378 FAX: (256) 381-9669

MEMBER MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

IAS ACCREDITED Metal Building Systems AC 472

CERTIFICATION

This is to certify that the metal building system components furnished by Bigbee Steel Buildings, Inc. are designed to comply with the design criteria indicated and applied as required by the building code shown below.

The building is designed in accordance with the latest applicable editions of AISC 360 Specification for Structural Steel Buildings, AISC 341 Seismic Provisions for Structural Steel Buildings, AISI S100 Standard North American Specification for the Design of Cold Formed Steel Structural Members, AWS D1.1 Structural Welding Code-Steel and the AWS D1.3 Structural Welding Code-Sheet Steel.

This certification is limited to the design of the structural framing and cladding components manufactured in Bigbee Steel Buildings, Inc. facility in Muscle Shoals, Alabama. This certification specifically excludes accessory items and those items not provided by Bigbee Steel Buildings, Inc. The building should be erected on a properly designed foundation and in accordance with the metal building erection drawings. The undersigned is not the engineer of record for the overall project.

Governing Building Code:	IBC 2018
Metal Building Dead Load:	Building Materials
Roof Live Load:	20 psf
Frame Live Load:	Reduced per Building Code
Collateral Load:	1 psf
Occupancy/Risk Category:	II
Ground Snow Load, Pg:	0 psf
Flat Roof Snow Load, Pf:	0 psf
Snow Load Importance, I:	1.00
Snow Exposure Factor, Ce:	1.0
Thermal Factor, Ct:	1.0
Snow Drifts:	Not applicable
Ultimate Wind Speed, Vult:	120 mph
Nominal Wind Speed, Vasd:	93 mph
Wind Exposure:	B
Wind Importance, I:	not applicable
Internal Pressure Coefficient, GCli:	±0.18 (Enclosed)
Seismic Importance, I:	1.0
Mapped Spectral Response Acceleration, Ss:	0.215
Mapped Spectral Response Acceleration, S1:	0.089
Site Class:	D
Design Spectral Response Accelerations, Sds:	0.229
Design Spectral Response Acceleration, Sd1:	0.142
Seismic Design Category:	C
Analysis Procedure:	Equivalent Lateral Force Procedure
Lateral Seismic Force Resisting System:	Steel systems not specifically detailed for seismic
Longitudinal Seismic Force Resisting System:	Steel systems not specifically detailed for seismic

	Lateral	Longitudinal
Design Base Shear, V:	4.80 kips	5.45 kips
Seismic Response Coefficient, Cs:	0.0765	0.0765
Response Modification Factor, R:	3.00	3.00

Additional Loads: No additional loads considered

JOB NUMBER: 16854

CUSTOMER NAME:	PINELAND CONTRACTORS INC
CUSTOMER LOCATION:	METTER GA
PROJECT NAME:	EVANS COUNTY PUBLIC WORKS
PROJECT LOCATION:	CLAXTON GA
PROJECT DESCRIPTION:	

☐ PRELIMINARY DRAWINGS (DO NOT USE FOR CONSTRUCTION)

☐ DRAWINGS FOR APPROVAL - DATED: mm/dd/yyyy
Drawings are for APPROVAL PURPOSES ONLY. After completing the section below, please return one (1) set. This project will NOT be scheduled for delivery until an approved set of drawings are returned to Bigbee Steel Buildings, Inc.

- ☐ No Exceptions Taken
- ☐ Make Corrections Noted
- ☐ Revise and Resubmit
- ☐ Not Accepted See Comments

Review is only for conformance with the design concept of the Project and compliance with the information given in the Contract documents.

Reviewed By: _____ Date: _____

☑ STRUCTURAL DRAWINGS
Drawings represent the primary structural framing being used on this project and depict the final design. Do NOT erect the building with these drawings. A final set of erection drawings will be supplied prior to and/or with delivery of the building showing piece marks and assembly details.

☐ FINAL ERECTION DRAWINGS
Drawings are for use in the field as erection documents.

ROOF SHEETING* / CLADDING — SEAM TYPE

☐ Bigbee Rib II	☑ 26ga	☐ 24ga			
☑ Bigbee Rib II PBR	☑ 26ga	☐ 24ga			
☐ BigbeeLok-324	☐ 24ga	☐ 22ga	☐ TripleLok	☐ QuadLok	☐ See Layout
☐ BigbeeVR	☐ 24ga	☐ 22ga	☐ TripleLok	☐ QuadLok	☐ See Layout
☐ Other: _____					

WALL SHEETING* / CLADDING

☐ Bigbee Rib II	☐ 26ga	☐ 24ga
☑ Bigbee Rib II PBR	☑ 26ga	☐ 24ga
☐ Bigbee Rib II RR	☐ 26ga	☐ 24ga
☐ Other: _____		

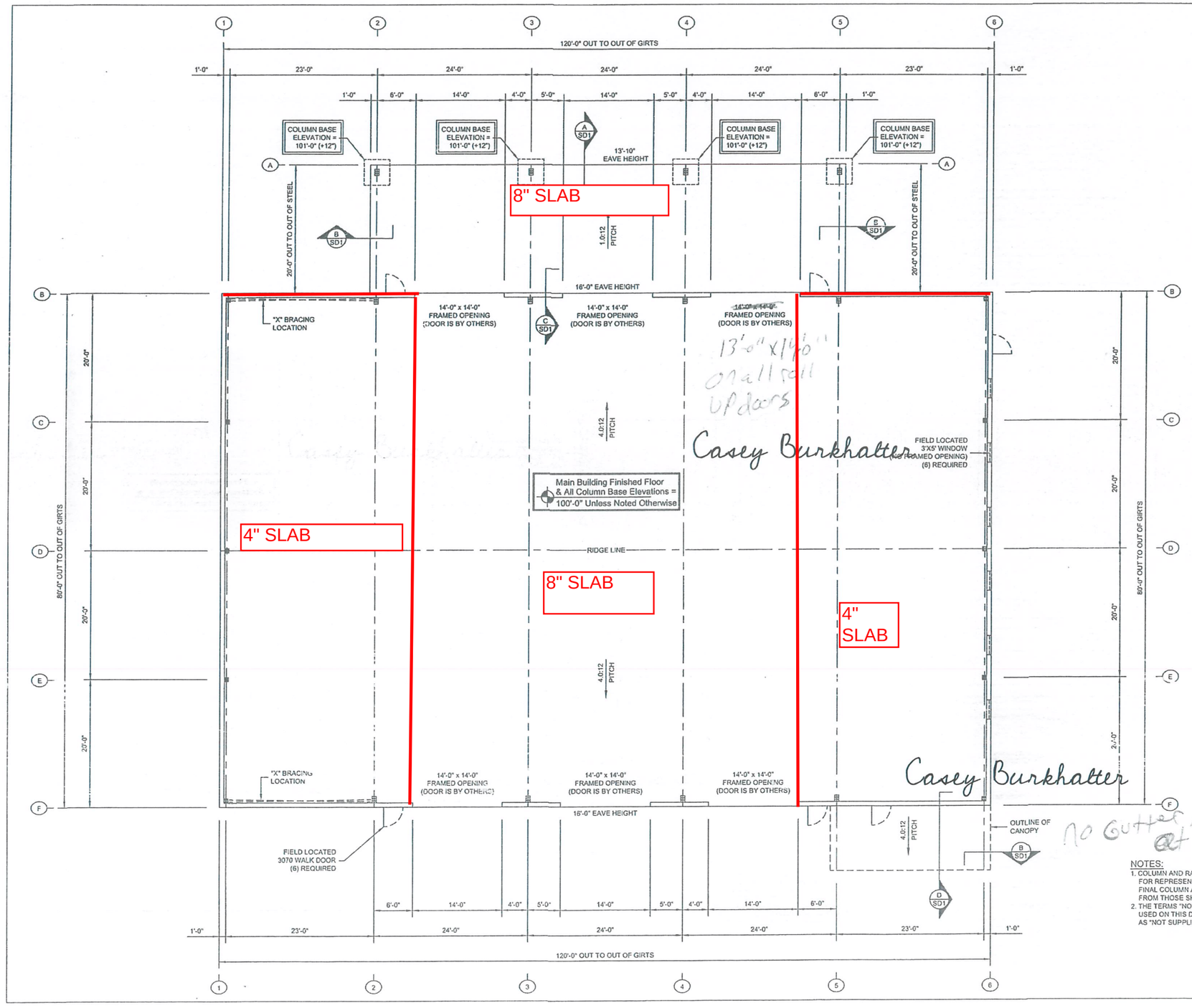
*Digital versions of all Bigbee Panel Profiles, Product Application Guides (including Technical Data), Installation & Seaming Guides are located on our website at: www.bigbee.com/products.htm

MOMENT FRAME A325 BOLT INSTALLATION**

☑ Snug Tight
☐ Pretension (Turn-of-Nut Method)
** See Drawing D-0 Note 1.5

DRAWING INDEX

GEORGIA REGISTERED No. PE015644 PROFESSIONAL ENGINEER WILLIAM S. FARNEMAN



NOTES:
1. COLUMN AND RAFTER SIZES SHOWN ARE FOR REPRESENTATIONAL PURPOSES ONLY. FINAL COLUMN AND RAFTER SIZES WILL VARY FROM THOSE SHOWN IN THIS DRAWING.
2. THE TERMS "NOT BY BIGBEE" OR "BY OTHERS" USED ON THIS DRAWING ARE TO BE INTERPRETED AS "NOT SUPPLIED BY BIGBEE STEEL BUILDINGS, INC."

BIGBEE
CUSTOM STEEL BUILDINGS
P.O. BOX 2314
MUSCLE SHOALS, ALABAMA 35662
PHONE(256) 383-7122 FAX(256) 381-4999

This drawing reflects preliminary steel layout plans and/or elevations, and sheeting and trim details for this structure. Its purpose is to clarify the project scope for which Bigbee Steel Buildings, Inc. is responsible, and to indicate our understanding of construction elements to be added by others. Construction elements depicted herein (beyond the metal building structure) are not to be construed as representing engineering design of those elements. Such engineering design must be secured from other design professionals.

The design of any elements added to the metal building structure by others does not fall under the engineering seal affixed to this drawing except as specifically stated in the Design Summary drawings. In addition, the design of the physical connection of such elements to the metal building structure is by others and does not fall under the engineering seal.

Nothing depicted in Bigbee Steel Building Inc. drawings is to be understood as providing architectural services. The services of a registered architect must be contracted by the owner to insure that a comprehensive code review is completed and that all local building code requirements are met. In particular, doors and windows shown in Bigbee Steel Buildings, Inc. drawings reflect only the customer's requests and have not been subjected to any building code review.

NOTE:
This plan is NOT to be used as an anchor bolt plan or as a substitute for erection drawings. Refer to the engineer sealed "Field Use" Anchor Bolt Plans for anchor bolt setting information & base plate details. Refer to the Erection Drawings shipped with the metal building for erection information. This drawing has been created to illustrate setup information only, and is NOT intended to be used in the field.

GEORGIA REGISTERED PROFESSIONAL ENGINEER JOHN T. STANLEY FARNHAM

REVISIONS

NO	DATE	BY	DESCRIPTION
1			
2			
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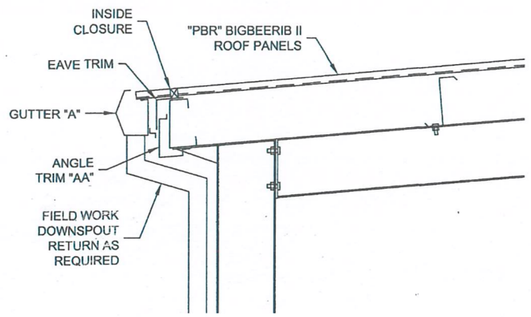
DESIGN CERTIFICATION
CERTIFICATION OF THE METAL BUILDING COMPONENTS SUPPLIED BY BIGBEE STEEL BUILDINGS, INC. IS LIMITED TO THE LOADS SPECIFIED ON THE BIGBEE STEEL BUILDINGS, INC. PURCHASE ORDER. THE BUILDING MUST BE ERECTED IN ACCORDANCE WITH THE ERECTION DRAWINGS AND EXCLUDES ITEMS NOT SUPPLIED BY BIGBEE STEEL BUILDINGS, INC. THE UNDERSIGNED IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT.

IAS ACCREDITED Metal Building Systems AC 472

MEMBER MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

DRAWING NAME: COLUMN LAYOUT
CUSTOMER: PINELAND CONTRACTORS METTER, GA
PROJECT: EVANS CO PUBLIC WORKS CLAXTON, GA
FILENAME: 16854 - CL1
JOB #: 16854
DRAWN: JTS
DESIGN:
CHECKED:
SCALE: NTS
APPROVED:
DATE: 08/30/23
DRAWING #: CL1



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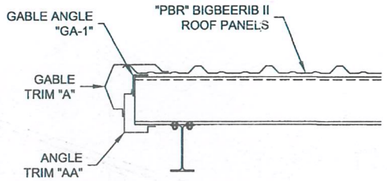

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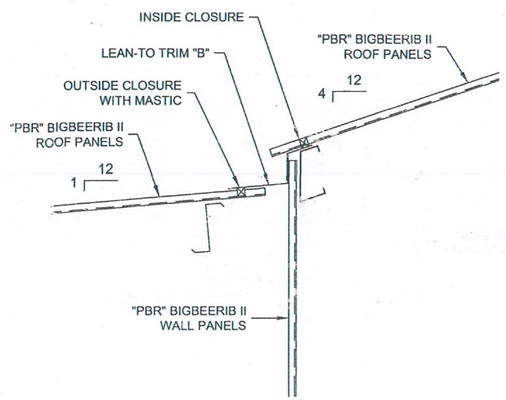
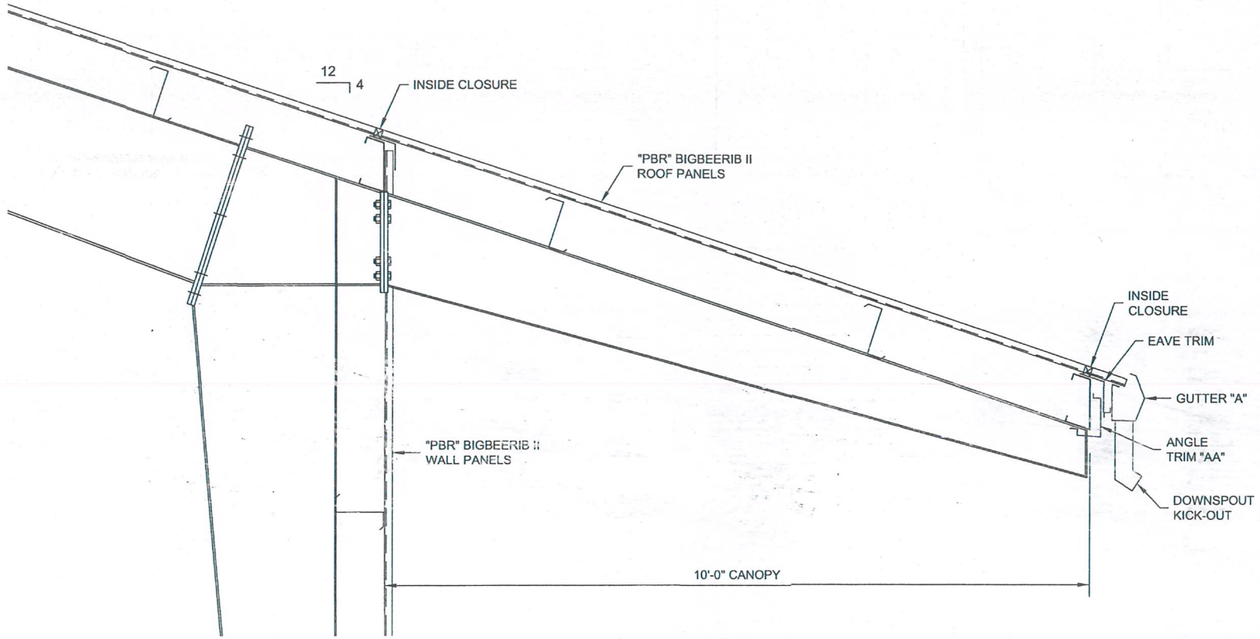


SECTION "C"



SECTION "D"

NOTES:

1. COLUMN AND RAFTER SIZES SHOWN ARE FOR REPRESENTATIONAL PURPOSES ONLY. FINAL COLUMN AND RAFTER SIZES WILL VARY FROM THOSE SHOWN IN THIS DRAWING.
2. THE TERM "BY OTHERS" USED ON THIS DRAWING IS TO BE INTERPRETED AS "BY OTHERS THAN BIGBEE STEEL BUILDINGS, INC."

BIGBEE
CUSTOM STEEL BUILDINGS
P.O. BOX 2314
MUSCLE SHOALS, ALABAMA 35662
PHONE (256) 383-7322 FAX (256) 381-9699

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NOTE:
THIS PLAN IS NOT TO BE USED AS AN ANCHOR BOLT PLAN OR AS A SUBSTITUTE FOR ERECTION DRAWINGS. REFER TO THE ENGINEER SEALED "FIELD USE" ANCHOR BOLT PLANS FOR ANCHOR BOLT SETTING INFORMATION & BASE PLATE DETAILS. REFER TO THE ERECTION DRAWINGS SHIPPED WITH THE METAL BUILDING FOR ERECTION INFORMATION. THIS DRAWING HAS BEEN CREATED TO ILLUSTRATE SETUP INFORMATION ONLY, AND IS NOT INTENDED TO BE USED IN THE FIELD.

GEORGIA REGISTERED PROFESSIONAL ENGINEER NO. PE019914 WILLIAM S. FARNEMAN 7/7/23

REVISIONS

NO	DATE	BY	DESCRIPTION
1			
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DESIGN CERTIFICATION

CERTIFICATION OF THE METAL BUILDING COMPONENTS SUPPLIED BY BIGBEE STEEL BUILDINGS, INC. IS LIMITED TO THE LOADS SPECIFIED ON THE BIGBEE STEEL BUILDINGS, INC. PURCHASE ORDER. THE BUILDING MUST BE ERECTED IN ACCORDANCE WITH THE ERECTION DRAWINGS AND EXCLUDES ITEMS NOT SUPPLIED BY BIGBEE STEEL BUILDINGS, INC. THE UNDERSIGNED IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT.

IAS ACCREDITED Metal Building Systems AC 472

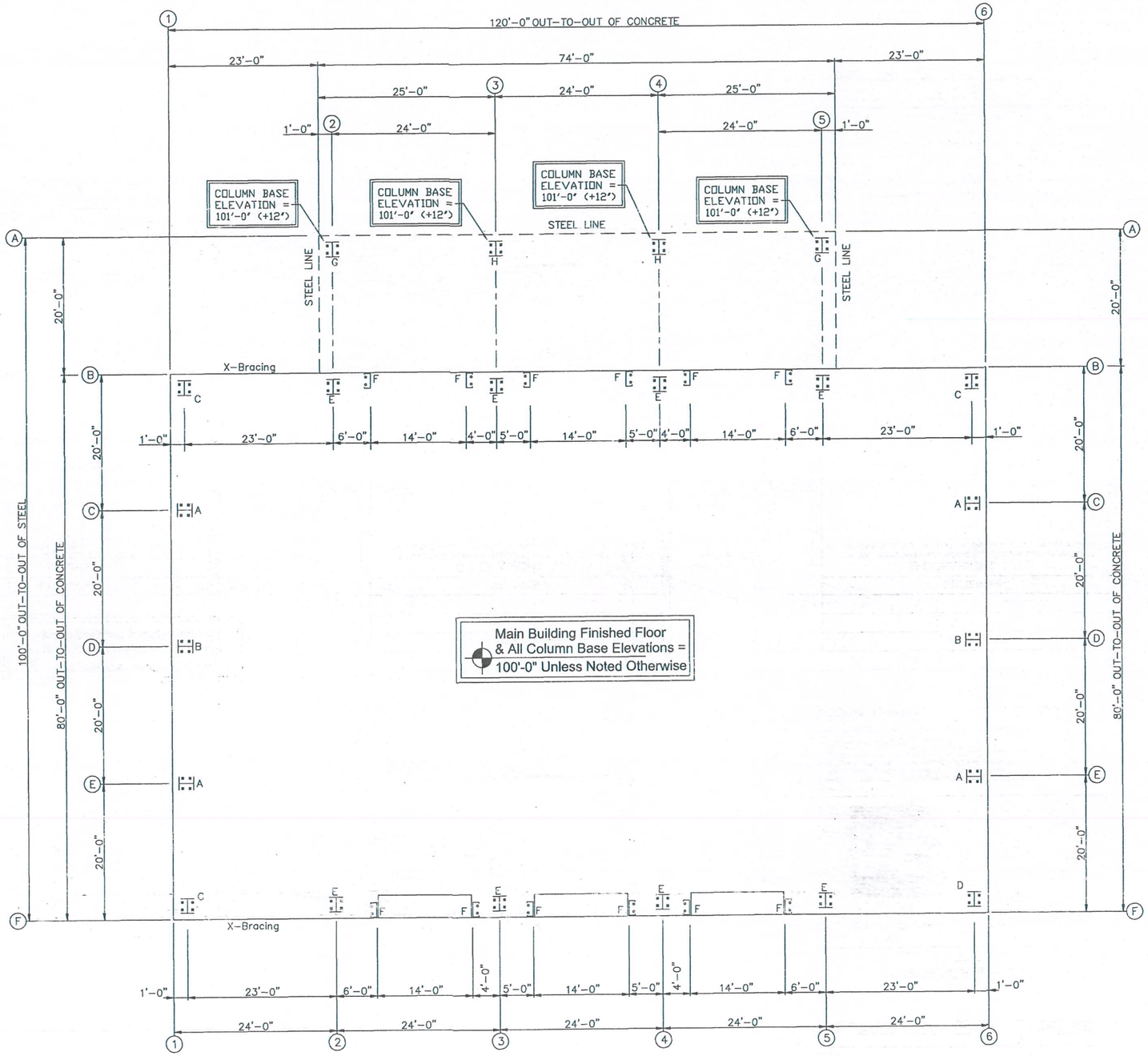
MEMBER MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

DRAWING NAME: SECTIONS AND DETAILS

CUSTOMER: PINELAND CONTRACTORS METTER, GA

PROJECT: EVANS CO PUBLIC WORKS CLAXTON, GA

FILENAME: 16854 - SD1		JOB #	16854
DRAWN JTS	DESIGN		DRAWING #
CHECKED	SCALE NTS		SD1
APPROVED	DATE 07/05/23		



ANCHOR BOLT PLAN

CAUTION:
REFER TO AB-2 FOR INDIVIDUAL BASE PLATE DETAILS. ANCHOR BOLT SETTING PATTERNS ARE NOT IDENTICAL AT EVERY COLUMN. CARE MUST BE EXERCISED TO INSURE BOLTS ARE PROPERLY SET AT EACH COLUMN LOCATION TO INSURE THAT THE BUILDING WILL ERECT PROPERLY.

← NOTE!

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Total Len (in)	Proj (in)
24	Jamb	1/2"	F1554 GA36	3.75	1.50
88	Frame	3/4"	F1554 GA36	12	2.50

BIGBEE
CUSTOM STEEL BUILDINGS
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MUSCLE SHOALS, ALABAMA 35662
PHONE:(256) 383-7322 FAX:(256) 381-9669

Component items such as (Roof Curbs, Roof Jacks/Boots, Roof Vents, Doors, Windows, Fans, Shutters, Louvers, etc.), when supplied by Bigbee Steel Buildings, Inc., MUST be installed using the Component Manufacturer's supplied installation instructions.

REVISIONS

NO	DATE	BY	DESCRIPTION
1			
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IAS ACCREDITED Metal Building Systems AC 472

GEORGIA REGISTERED PROFESSIONAL ENGINEER

MEMBER MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

DRAWING NAME:
ANCHOR BOLT PLAN

CUSTOMER:
PINELAND CONTRACTORS
METTER, GA

PROJECT:
EVANS CO PUBLIC WORKS
CLAXTON, GA

FILENAME: 16854 - AB-Plans ANDVGI.dxf

JOB # 16854

DRAWN

DESIGN SF

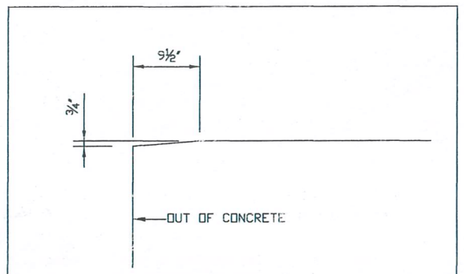
CHECKED JTS

SCALE NONE

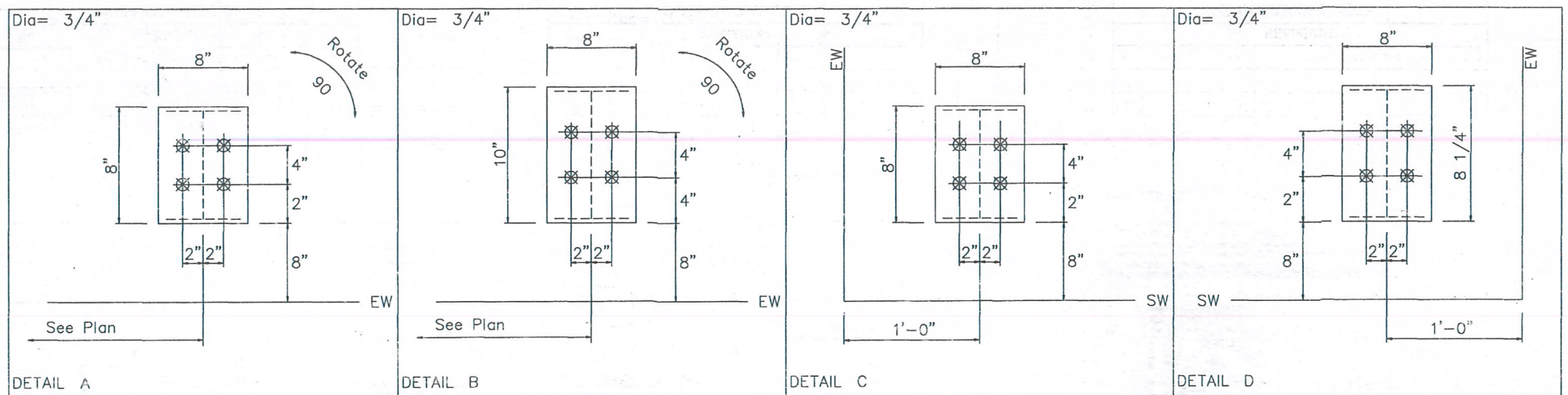
DRAWING # AB1

APPROVED

DATE



SLAB SLOPE DETAIL @ OVERHEAD DOOR



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REVISIONS

NO	DATE	BY	DESCRIPTION
1			
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IAS ACCREDITED Metal Building Systems AC 472

GEORGIA REGISTERED No. PE035911 PROFESSIONAL ENGINEER M. A. FARWELL

M E M B E R MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

DRAWING NAME:
BASE PLATE DETAILS

CUSTOMER:
PINELAND CONTRACTORS
METTER, GA

PROJECT:
EVANS CO PUBLIC WORKS
CLAXTON, GA

FILENAME:	JOB #
16854 - AB-Plans	16854
DRAWN	DESIGN SP
CHECKED JTS	SCALE NONE
	DRAWING # AB2

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width (ft)	= 80.0
Length (ft)	= 120.0
Eave Height (ft)	= 16.0/ 16.0
Roof Slope (rise/12)	= 4.0/
Dead Load (psf)	= 2.5
Collateral Load (psf)	= 1.0
Roof Live Load(psf)	= 20.0
Frame Live Load(psf)	= 12.0
Wind Speed (mph)	= 120.0
Wind Code	= GSBC 20 (IBC 18)
Exposure	= B
Closure	= Enclosed
Importance Wind	= 1.00
Importance Seismic	= 1.00
Seismic Zone	= C
Seismic Coeff (Fa*Ss)	= 0.34

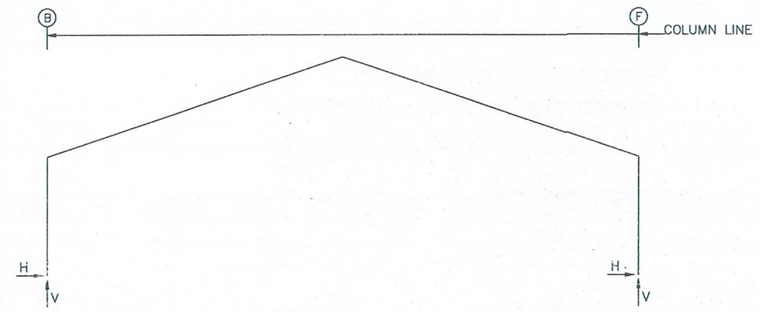
ID	Description
1	Dead+Collateral+Live
2	0.6Dead+0.6Wind_Left1
3	0.6Dead+0.6Wind_Right1
4	0.6Dead+0.6Wind_Long1L
5	0.6Dead+0.6Wind_Long2L
6	0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
7	Dead+Collateral+0.75Live+0.45Wind_Right2+0.45Wind_Suction
8	0.6Dead+0.6Wind_Suction+0.6Wind_Long1L
9	Dead+Collateral+E1PAT_LL_1
10	0.6Dead+0.6Wind_Right1+0.6Wind_Suction
11	0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
12	Dead+Collateral+E1PAT_LL_2
13	0.6Dead+0.6Wind_Suction+0.6Wind_Long2L
14	Dead+Collateral+E1PAT_LL_3
15	Dead+Collateral+0.75Live+0.45Wind_Left2+0.45Wind_Suction
16	Dead+Collateral+E2PAT_LL_5
17	0.6Dead+0.6Wind_Left1+0.6Wind_Suction
18	Dead+Collateral+E2PAT_LL_2
19	Dead+Collateral+E2PAT_LL_3
20	Dead+Collateral+E2PAT_LL_4

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel_Shear (lb/ft) Wind	Seis
L_EW	1						38	7
F_SW	F	1,2	8.1	4.7	2.3	1.4		
R_EW	6						45	8
B_SW	B	2,1	9.9	5.7	3.1	1.8		

Reactions for seismic represent shear force, Eh

FRAME LINES: 2 3 4 5



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Bolt(in) Dia	Base_Plate(in) Width	Base_Plate(in) Length	Base_Plate(in) Thick	Grout (in)
2*	B	1	10.1	20.3	2	−6.5	−10.4	4	0.750	8.000	12.25	0.500	0.0
					4	0.1	−12.3						
2*	F	3	6.2	−10.5	1	−10.2	19.5	4	0.750	8.000	12.25	0.500	0.0
		1	−10.2	19.5	3	6.2	−10.5						

2* Frame lines: 2 3 4 5

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert	Wind_Left2 Horiz	Wind_Left2 Vert
2*	B	2.2	4.6	0.7	1.2	7.2	14.5	−13.0	−21.9	−2.8	−16.0	−12.0	−13.0
2*	F	−2.3	4.2	−0.7	1.0	−7.3	14.3	3.9	−15.2	12.6	−21.7	1.3	−7.3

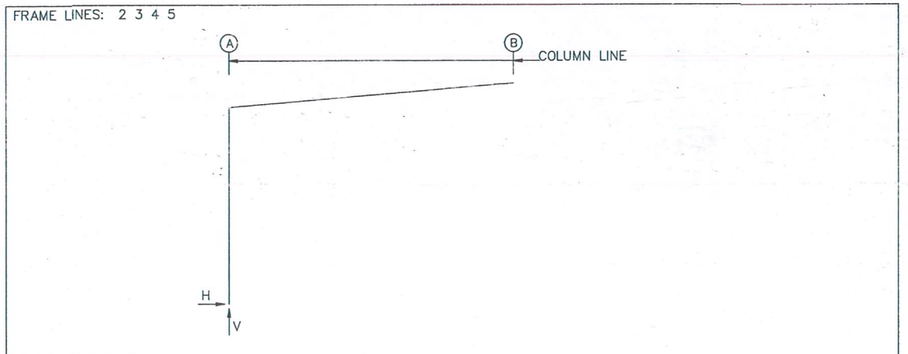
Frame Line	Column Line	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert	Seismic_Long Horiz	Seismic_Long Vert
2*	B	−1.8	−7.1	−2.0	−25.0	−4.3	−22.8	−0.6	−0.2	0.6	0.2	−0.2	−1.9
2*	F	10.0	−13.9	8.1	−19.2	5.8	−21.4	−0.5	0.2	0.5	−0.2	−0.1	−1.3

2* Frame lines: 2 3 4 5

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Bolt(in) Dia	Base_Plate(in) Width	Base_Plate(in) Length	Base_Plate(in) Thick	Grout (in)
1	B	4	0.0	−1.2	6	−5.9	−4.7	4	0.750	8.000	8.000	0.375	0.0
		7	0.0	4.4									
1	C	8	2.6	−3.9	6	−2.4	−3.9	4	0.750	8.000	8.000	0.375	0.0
		9	0.0	7.1	8	2.6	−3.9						
1	D	10	3.5	−1.6	11	−3.2	−0.9	4	0.750	8.000	10.00	0.375	0.0
		12	0.0	7.0	10	3.5	−1.6						
1	E	13	2.6	−3.9	11	−2.4	−3.9	4	0.750	8.000	8.000	0.375	0.0
		14	0.0	7.1	13	2.6	−3.9						
1	F	5	0.0	−1.2	11	−4.9	−4.1	4	0.750	8.000	8.000	0.375	0.0
		15	0.0	3.9									
6	F	6	0.9	−3.7	16	−1.7	6.2	4	0.750	8.000	8.250	0.375	0.0
		1	−0.9	9.1	17	0.3	−5.7						
6	E	8	2.6	−3.5	6	−2.4	−3.5	4	0.750	8.000	8.000	0.375	0.0
		18	0.0	6.2	8	2.6	−3.5						
6	D	10	3.5	−1.8	6	−3.2	−1.2	4	0.750	8.000	10.00	0.375	0.0
		19	0.0	7.4	10	3.5	−1.8						
6	C	13	2.6	−3.9	11	−2.4	−3.9	4	0.750	8.000	8.000	0.375	0.0
		20	0.0	7.1	13	2.6	−3.9						
6	B	5	0.0	−1.2	5	0.0	−1.2	4	0.750	8.000	8.000	0.375	0.0
		16	0.0	2.8									

FRAME LINES: 2 3 4 5



RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert	Wind_Left2 Horiz	Wind_Left2 Vert
2*	A	0.0	0.6	0.0	0.1	0.1	2.6	−0.4	−2.6	0.7	−1.7	−1.0	−1.4
3*	A	0.0	0.9	0.0	0.3	0.1	3.6	−0.8	−4.7	1.3	−3.2	−1.8	−2.6

Frame Line	Column Line	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert
2*	A	0.2	−0.6	1.0	−2.7	1.0	−1.7
3*	A	0.3	−1.1	1.8	−5.0	1.8	−3.3

2* Frame lines: 2 5
3* Frame lines: 3 4

BIGBEE
CUSTOM STEEL BUILDINGS
P.O. BOX 2314
MUSCLE SHOALS, ALABAMA 35662
PHONE:(256) 383-7322 FAX:(256) 381-9869

Component items such as (Roof Curbs, Roof Jacks/Boots, Roof Vents, Doors, Windows, Fans, Shutters, Louvers, etc.), when supplied by Bigbee Steel Buildings, Inc., MUST be installed using the Component Manufacturer's supplied Installation Instructions.

REVISIONS

NO	DATE	BY	DESCRIPTION
1			
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DESIGN CERTIFICATION

CERTIFICATION OF THE METAL BUILDING COMPONENTS SUPPLIED BY BIGBEE STEEL BUILDINGS, INC. IS LIMITED TO THE LOADS SPECIFIED ON THE BIGBEE STEEL BUILDINGS, INC. PURCHASE ORDER. THE BUILDING MUST BE ERECTED IN ACCORDANCE WITH THE ERECTION DRAWINGS AND EXCLUDES ITEMS NOT SUPPLIED BY BIGBEE STEEL BUILDINGS, INC. THE UNDERSIGNED IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT.

IAS ACCREDITED Metal Building Systems AC 472

GEORGIA REGISTERED No. PE015514 PROFESSIONAL ENGINEER JS FARNEMAN

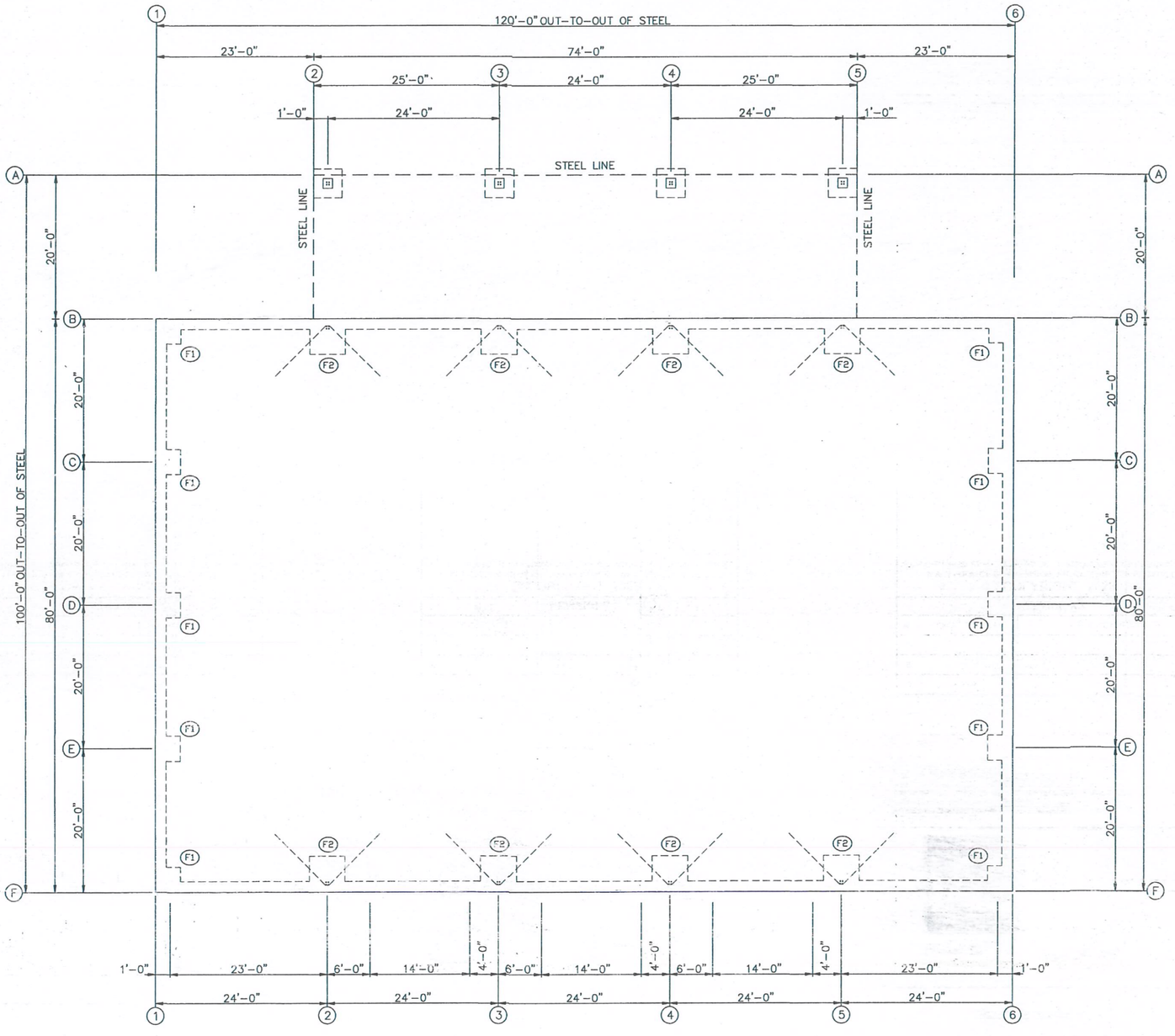
MEMBER MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

DRAWING NAME:
COLUMN & BRACING REACTIONS

CUSTOMER:
PINELAND CONTRACTORS
METTER, GA

PROJECT:
EVANS CO PUBLIC WORKS
CLAXTON, GA

FILENAME: 16854 - AB-Plans	JOB # 16854	
DRAWN	DESIGN SF	DRAWING # AB3
CHECKED JTS	SCALE NONE	
APPROVED	DATE	



This footing design is based on reasonable and conservative **assumptions** made by the engineer. No Geotechnical Report was provided prior to generating this design. It is the owner's responsibility to insure that the assumptions upon which this design is based are in compliance with the Geotechnical Report. If they are determined to contradict, then this design is invalid.

CONCRETE SLAB MUST BE IN PLACE PRIOR TO ERECTION OF STEEL STRUCTURE TO ENGAGE HAIRPINS.

SLAB CONTRACTION JOINTS ARE REQUIRED. SEE NOTES FOR GUIDANCE ON JOINT SPACING AND CUTTING.

REFER TO THE STANDARD FOOTING SCHEDULE(S) SUPPLIED WITH THIS PACKAGE FOR FOOTING DESIGNATION REFERENCE INFORMATION.

SLAB PERIMETER IS REQUIRED TO HAVE AN APPROPRIATE TURNDOWN AS SHOWN IN THE FOOTING SCHEDULE(S).

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Component items such as (Roof Curbs, Roof Jacks/Boots, Roof Vents, Doors, Windows, Fans, Shutters, Louvers, etc.), when supplied by Bigbee Steel Buildings, Inc., MUST be installed using the Component Manufacturer's supplied Installation Instructions.

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IAS ACCREDITED Metal Building Systems AC 472

GEORGIA REGISTERED PROFESSIONAL ENGINEER WILLIAM S. FARTHEMAN

MEMBER MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

DRAWING NAME:
FOOTING PLAN

CUSTOMER:
PINELAND CONTRACTORS
METTER, GA

PROJECT:
EVANS CO PUBLIC WORKS
CLAXTON, GA

FILENAME: 16854 - AB-Plans ANDWGI.dxf		JOB #	16854
DRAWN		DESIGN SF	DRAWING # F1
CHECKED JTS		SCALE NONE	

Basic Building Anchorage Notes

Footing designations shown on the Footing Plan require:	
1. Minimum Soil Bearing Capacity of:	**2000** psf.
2. Maximum Frost Depth of:	**20** in.

The footing and foundation information shown on these pages is provided as part the total building package. The sections and dimensional sizes shown are suitable to properly anchor the building structure IF THE ABOVE DESIGN CRITERIA IS VALIDATED BY THE GEOTECHNICAL REPORT.

Earthwork and site preparation advice is not included in the scope of this design. The owner/contractor is referred to the guidance of the site-specific geotechnical report. The footing/slab design presented herein assumes that the site has been fully prepared in accordance with the requirements of the geotechnical report and under the direction of geotechnical engineers.

Footing and slab construction is also to proceed according to the requirements of the geotechnical engineering report specific to this particular project. This report must be reviewed by the installer prior to proceeding. Where the geotechnical report conflicts with these notes, the geotechnical report takes precedence. Should any part of this foundation design appear to contradict with the geotechnical report, then the foundation designer must be notified prior to proceeding with construction.

Other foundation elements beyond and unrelated to the basic building anchorage, or other foundation elements that conflict with the basic building anchorage, may invalidate the information shown here and will require the services of a local design professional. Such elements might include, but are not limited to, brick or block wall system foundations, loading docks, retaining walls, structural slabs, special floor constructions, etc.

The building anchors are to be placed per the Anchor Plan drawing (typically AB-1). The owner/contractor should carefully review the overall dimensions of the slab, the frame line dimensions, and the individual base plate details prior to setting the anchors. The anchors must be installed exactly to the requirements of the Anchor Plan drawing for the building to be properly anchored and erected. Anchor bolts/rods should conform to ASTM F1554 grade 36.

The slab thickness shown in the footing and foundation information is illustrative only. The owner/contractor must consider the slab loading and usage in order to determine the slab thickness.

A base course (minimum of 4"thick with maximum particle size of 1") of clean, crushed stone to provide drainage and stability is recommended on top of the prepared site. A 6-mil polyethylene vapor retarder with joints lapped not less than 6 inches is recommended between the base course and the concrete floor slab.

Foundation reinforcement steel should be placed according to the following code requirements:

Footing mat placements and other reinforcing bars in concrete cast against earth:	3" clear
Otherwise:	1½"-2" clear

Perimeter reinforcing bars (rebar in slab turndowns) are to be continuous. Splicing is to be per ACI-318. Minimum splice lengths are to be as follows:

17" for #3 bars,
22" for #4 bars,
28" for #5 bars,
33" for #6 bars.

Slab penetration points (such as interior column locations and pipe penetrations) should be isolated from the slab. Additional slab reinforcing should be provided around such locations as well as perimeter reentrant points to inhibit slab cracking.

Temperature and shrinkage reinforcement is required and the following options are listed in order of considered effectiveness:

Deformed bars: (ASTM A 615 grade 60 reinforcing steel):
4" slab--#3 reinforcing bars at 24" centers each way
6" slab--#3 reinforcing bars at 18" centers each way
8" slab--#3 reinforcing bars at 12" centers each way

Welded wire reinforcing (ASTM A 185):
4" slab--6x6-w1.4xw1.4
6" slab--6x6-w1.4xw1.4
8" slab--6x6-w2.0xw2.0

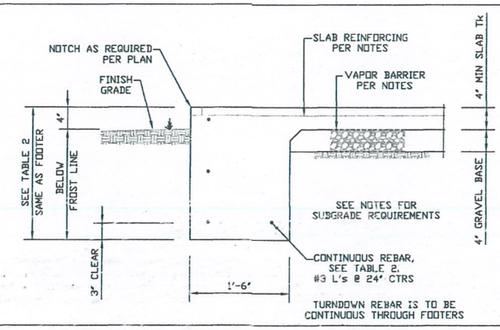
Shrinkage and temperature reinforcement should be at or above mid-depth of the slab on grade. Placement 1" to 2" below the top surface of the concrete is common.

Polypropylene or nylon fibers added to concrete at a rate of ¾ to 1 ½ pounds per cubic yard of concrete serve to control shrinkage during curing but are not considered acceptable as substitutes for the temperature and shrinkage reinforcement options listed above.

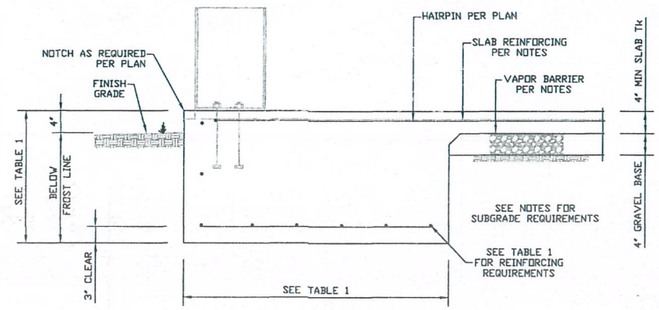
Slab contraction joints are required. Contraction joint recommended spacing is (in feet) no more than 2-3 times the slab thickness (in inches), with joint spacing not exceeding 15'. Joints must be cut to 25% of the depth of the slab within 4-12 hours after finishing the concrete. Cuts are not to sever the slab reinforcing steel. Cuts should occur at all column centerlines in both directions. Cuts should be straight lines, incorporate reentrant points, and not form T's.

Dowel joints are required at all construction joints, and at contraction joints in slabs subject to moving loads. Dowels should be saw cut (not sheared) and must be installed straight and aligned. The dowels should be greased or cased at one end to enable slippage. Formed keyways at construction joints are not recommended.

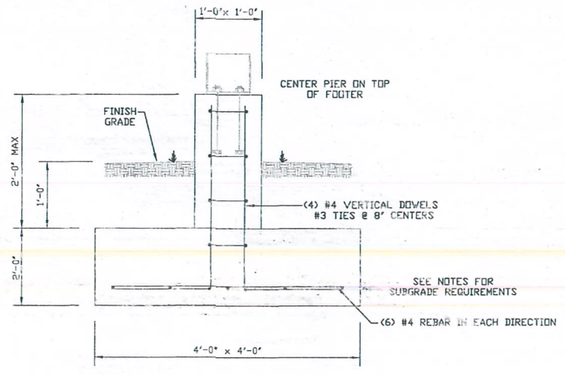
Concrete recommendation:
Cement: ASTM C 150 type I or II
ASTM C33 aggregate: size 57
Target Air: 4.5% (+/-1.5% Tolerance)
Water/cement ratio (by weight): 0.55 max.
Compressive strength at 28 days: 4000 psi
Maximum slump: 3"+- 1"



MONOLITHIC POUR TURNDOWN

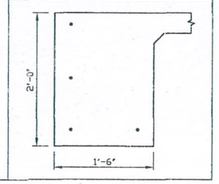


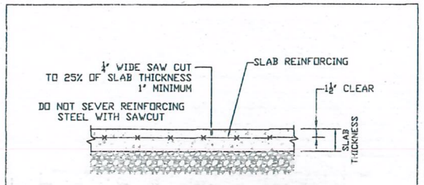
MONOLITHIC POUR COLUMN FOOTER



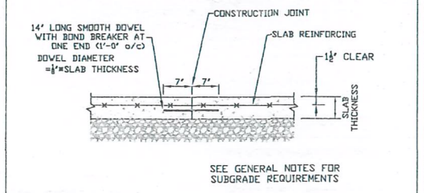
LEAN TO FOOTER WITH PIER

TABLE 1: FOOTER DIMENSIONS & REINFORCING			
	SQUARE FOOTER DIMENSIONS	FOOTER DEPTH	BOTTOM REINFORCING (EACH DIRECTION)
F1	3'-6"x3'-6"	2'-0"	(8) #4x36" LONG
F2	5'-0"x5'-0"	2'-0"	(7) #5x54" LONG

TABLE 2: TURNDOWN REBAR		
TURNDOWN DEPTH	REBAR COUNT	REBAR PLACEMENT
2'-0"	(4) #4	

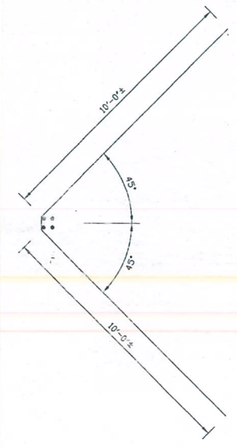


CONTRACTION JOINT DETAIL



CONSTRUCTION JOINT DETAIL

HAIRPIN REQUIREMENTS



#4 x 20'-0" HAIRPIN: BEND AT 45 deg. AROUND BOLTS AND EXTEND INTO SLAB AT MID-DEPTH. (TYPICAL WHERE SHOWN ON PLAN)

BIGBEE
CUSTOM STEEL BUILDINGS
P.O. BOX 2314
MUSCLE SHOALS, ALABAMA 35662
PHONE (256) 383-7322 FAX (256) 381-9669

GEORGIA REGISTERED No. PE015914 PROFESSIONAL ENGINEER WILLIAM S. FARNELL

REVISIONS			
NO	DATE	BY	DESCRIPTION
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DESIGN CERTIFICATION

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IAS ACCREDITED Metal Building Systems AC 472

MEMBER MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

DRAWING NAME:
STANDARD FOOTING SCHEDULE

CUSTOMER:
PINELAND CONTRACTORS
METTER, GA

PROJECT:
EVANS CO PUBLIC WORKS
CLAXTON, GA

FILENAME: 16854 - AB-Plans		JOB #	16854
DRAWN		DESIGN SF	DRAWING # S-1
CHECKED JTS		SCALE	
APPROVED		DATE 07/12/23	

GENERAL NOTES

1.1 Fabrication shall be in accordance with Bigbee Steel Buildings, Inc. standard practices in compliance with the applicable sections relating to design requirements and allowable stresses of the latest edition of the "AWS Structural Welding Codes - D1.1 and D1.3".

MATERIALS	ASTM DESIGNATION *	MINIMUM YIELD
Hot rolled mill shapes	A992	FY = 50 KSI
Structural steel plate	A529/A572/A1011/A1018	FY = 55 KSI
Cold formed light gauge shapes	A1011	FY = 55 KSI
Cable bracing	A475	Extra High Strength
Roof and wall sheets	A792	FY = 50, 80, KSI
Machine bolts	A307, gr A	FY = 36 KSI
High strength bolts (½" Ø to 1" Ø)	A325, type 1	FY = 120 KSI
High strength bolts (⅛" Ø to ½" Ø)	A325, type 1	FY = 105 KSI
Anchor bolts (if supplied)	A307, gr C, F1554	FY = 36 KSI
Pipe	A500, gr B	FY = 42 KSI

* LATEST ISSUE

1.3 PRIMER
Framing members shall be cleaned and prepared in accordance with SSPC-SP2 as a minimum, and primer coated with Bigbee Steel Buildings, Inc.'s standard Red Oxide color meeting SSPC No. 15. The shop coat of paint is the prime coat of the protective system. It is intended as protection for only a short period of exposure in ordinary atmospheric conditions and is considered a temporary and provisional coating. Bigbee Steel Buildings, Inc. is not responsible for any deterioration of the shop primer paint as a result of improper handling and / or storage. Bigbee Steel Buildings, Inc. shall not be responsible for any field applied paint and / or coatings. (Section 6.5 AISC Code of Standard Practice, 13th Edition).

1.4 GALVANIZED OR SPECIAL COATINGS
See Contract Documents.

1.5 A325 BOLT TIGHTENING REQUIREMENTS

Snug Tight condition is defined as the tightness that exists when all plies in a joint are in firm contact. This may be attained by a few impacts of an impact wrench or the full effort of a person using an ordinary spud wrench. Hardened washers are not required except when bolts are installed in oversized or slotted holes.

Turn-of-Nut is the method to be used for connections requiring full pre-tensioning. There shall first be enough bolts brought to a snug tight condition to insure that the plies of the joint are in firm contact. Bolts shall then be placed in remaining holes and the connection shall be tightened additionally by the applicable amount of nut rotation specified below, with tightening progressing systematically from the most rigid part of the joint to its free edges. During this operation there shall be no rotation of the part not turned by the wrench.

A325 BOLTS

BOLT SIZE IN INCHES	WIDTH IN INCHES	SPECIFIED MINIMUM FASTENER TENSION KIPS (1000 Lbs)	SPECIFIED NUT ROTATION	
			BOLT LENGTH ≤ 4 DIAMETERS	BOLT LENGTH > 4 DIAMETERS
1/2"	7/8"	12	1/3 TURN	1/2 TURN
3/4"	1 1/4"	28		
7/8"	1 7/16"	39		
1"	1 5/8"	51		

1.6 CLOSURE STRIPS ARE FURNISHED FOR APPLICATION
INSIDE - Under roof panels at eave.
OUTSIDE - Under continuous ridge vent skirts.

NOTE: Conditions vary at hips, valleys, fascias, mansards and canopies refer to Erection Drawings for closure applications.

1.7 ERECTION NOTE
All bracing strapping and bridging shown and provided by Bigbee Steel Buildings, Inc. for this building is required and shall be installed by the erector as a permanent part of the structure unless noted otherwise on the erection drawing. If additional bracing is required for stability during erection, it shall be the erector's responsibility to determine the amount of such bracing and to procure and install as needed.

1.8 UNLOADING AND ERECTION NOT BY BIGBEE STEEL BLDG., INC.
Due to Bigbee Steel Buildings, Inc.'s insurance policy, Bigbee Steel Buildings, Inc. drivers MAY NOT ASSIST in the unloading of structure.

1.9 CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
Claims for correction of alleged misfits will be disallowed unless Bigbee Steel Buildings, Inc. has received prior notice thereof and allowed reasonable inspection of such misfits. The correction of minor misfits by the use of drift pins to draw the components into line, shimming, moderate amounts of reaming, chipping and cutting, and the replacement of minor shortages of material are a normal part of erection and are not subject to claim. No part of the building may be returned for alleged misfits without the prior approval of Bigbee Steel Buildings, Inc..

BUYER / END USE CUSTOMER RESPONSIBILITIES

2.1 It is the responsibility of the BUYER / USE CUSTOMER to obtain appropriate approvals and secure necessary permits from City, County, State or Federal Agencies as required, and to advise / release Bigbee Steel Buildings, Inc. to fabricate upon receiving such.

2.2 Bigbee Steel Buildings, Inc. standard specifications apply unless stipulated otherwise in the Contract Documents. Bigbee Steel Buildings, Inc.'s design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work with any other interpretations to the contrary not withstanding. It is understood by both Parties that the BUYER / END USE CUSTOMER is responsible for clarification of inclusions or exclusions from the architectural plans and / or specifications.

2.3 In case of discrepancies between Bigbee Steel Buildings, Inc.'s structural steel plans or other trades, Bigbee Steel Buildings, Inc.'s plans govern. (Section 3 AISC Code of Standard Practices, 13th Edition).

2.4 Approval of Bigbee Steel Buildings, Inc. drawings and calculations indicates that Bigbee Steel Buildings, Inc. has correctly interpreted and applied the Contract Documents. The approval constitutes the contractor / owners acceptance of Bigbee Steel Buildings, Inc.'s design concepts, assumptions, and loading. (Section 4.4 AISC Code of standard practices 13th edition, and section 3.3.3 MBMA 2006 Metal Building Systems Manual).

2.5 Once the BUYER / END USE CUSTOMER has signed Bigbee Steel Buildings, Inc.'s Approval Package and the project is released for fabrication, changes shall be billed to the BUYER / END USE CUSTOMER including material, engineering, and other costs. An additional fee may be charged if the project must be moved from the fabrication and shipping schedule.

2.6 The BUYER / END USE CUSTOMER is responsible for overall project coordination. All interface, compatibility, and design considerations concerning any materials not furnished by Bigbee Steel Buildings, Inc. are to be considered and coordinated by the BUYER / END USE CUSTOMER. Specific design criteria concerning this interface between materials must be furnished before release for fabrication or Bigbee Steel Buildings, Inc.'s assumptions will govern (Section 4 and Commentary AISC Code of Standard Practice, 13th Edition).

2.7 It is the responsibility of the BUYER / END USE CUSTOMER to insure that Bigbee Steel Buildings, Inc.'s plans comply with applicable requirements of any governing building authorities. The supplying of sealed engineering data and drawings for the metal building system does not imply or constitute an agreement that Bigbee Steel Buildings, Inc. or its design engineers are acting as the engineer of record or design professional for a construction project. These drawings are sealed only to certify the design of the structural components furnished by Bigbee Steel Buildings, Inc.

2.8 The BUYER / END USE CUSTOMER is responsible for setting anchor bolts and erection of steel in accordance with Bigbee Steel Buildings, Inc.'s "For Construction" drawing only. Temporary supports such as guys, braces, falsework, cribbing or other elements required for erection operation shall be determined and furnished and installed by the erector. No items should be purchased from a preliminary set of drawings, including anchor bolts. Use only final "FOR CONSTRUCTION DRAWINGS" for this use. (Section 7 AISC Code of Standard Practices, 13th Edition).

2.9 Bigbee Steel Buildings, Inc. is responsible for the design of the anchor bolts to permit the transfer of force between the base plate and anchor bolts in shear, bearing and tension, but is not responsible for the transfer of anchor bolts forces to concrete or the adequacy of the anchor bolts in relation to the concrete.
Unless otherwise provided in the Order Documents, Bigbee Steel Buildings, Inc. does not design and is not responsible for the design, material, and construction of the foundation or foundation embedments. The BUYER / END USE CUSTOMER should assure himself that adequate provisions are made in the foundation design for loads imposed by column reactions of the building, other imposed loads, and bearing capacity of the soil and other conditions of the building site.
It is recommended that the anchorage and foundation of the building be designed by a Registered Professional Engineer experienced in the design of such structures (Section A3 MBMA 2006 Metal Building Systems Manual).

2.10 Normal erection operations include the corrections of minor misfits by moderate amounts of reaming, chipping, welding or cutting, and the drawing of elements into line through the use of drift pins. Errors which cannot be corrected by the foregoing means or which require major changes in member configuration are to be reported immediately to Bigbee Steel Buildings, Inc. by the BUYER / END USE CUSTOMER to enable whoever is responsible either to correct the error or to approve the most efficient and economical method of correction to be used by others. (Section 7.12 AISC Code of Standard Practices, 13th Edition).

2.11 It is not permissible to cut, drill or alter the metal building components supplied by Bigbee Steel Buildings, Inc. unless such work is clearly specified in the contract documents or verified in writing by Bigbee Steel Buildings, Inc. Whenever such work is specified, the BUYER / END USE CUSTOMER is responsible for furnishing complete information as to materials, size, location and number of alterations prior to preparation of shop drawings. (Section 7 AISC Code of Standard Practice, 13th Edition).

2.12 WARNING
In no case should Aluminized Zinc steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosive effects on the Aluminized Zinc alloy coating when they are in contact with Aluminized Zinc steel panels. Even run-off from copper flashing, wiring or tubing onto Aluminized Zinc should be avoided.

2.13 SAFETY COMMITMENT
Bigbee Steel Buildings, Inc. has a commitment to manufacture quality building components that can be safely erected. However, the safety commitment and job site practices of the erector are beyond the control of Bigbee Steel Buildings, Inc. It is strongly recommended that safe working conditions and accident prevention practices be the top priority of any job site. Local, State and Federal safety and health standards should always be followed to help insure worker safety. Make certain all employees know the safest and most productive way of erecting a building.
Emergency procedures should be known to all employees.
Daily meetings highlighting safety procedures are also recommended. The use of hard hats, rubber sole shoes for roof work, proper equipment for handling material, and safety nets where applicable, are recommended.

SERVICEABILITY CONSIDERATIONS

3.1 Deflections of buildings and building components are part of the overall serviceability criteria that must be carefully considered by the BUYER / END CUSTOMER for a building project. The AISC Code of Standard Practice stipulates that when a fabricator has both design and fabrication responsibility, the owner must provide the "performance criteria of the structural steel frame". Absent of this criteria it is the design practice of Bigbee Steel Buildings, Inc. to use the "*Serviceability Design Considerations for Steel Buildings*", AISC Design Guide #3, as a reference for minimum standards.

ATTACHING BUILDING MATERIALS AND COMPONENTS TO THE METAL BUILDING SYSTEM

4.1 Special care and detailing should be considered when attaching building materials and components not designed and supplied by Bigbee Steel Buildings, Inc. to the metal building system. In areas where snow and ice accumulations can occur vertical deflections of the structural steel frames and purlins can be an issue when:

4.1a Attaching stud walls directly to the metal building framing without allowing for connection slip at the wall / building interface. Even a minimal amount of vertical deflection (+/- 3/8") of the structural steel framing can cause a metal stud to deflect laterally 5" to 6" when no allowance for building deflection has been made.

4.1b Suspending ceilings from the structural frames or purlins. In hallways or small rooms the ceiling should be supported from permanent walls or partitions. In larger rooms where the ceilings must be suspended from the metal building system it is important to allow the ceiling to deflect at the outside walls at the same rate it deflects in the center of the room. If the ceiling is supported continuously along any non-yielding wall while the rest of the ceiling is deflecting, obvious problems occurs.

BUILDING POROSITY

5.1 Bigbee Steel Buildings, Inc. assumes that all overhead doors and all windows and walk doors which are not furnished by Bigbee Steel Buildings, Inc., have been designed to resist the required wind load of your building. In many cases, openings which have not been designed for full wind load will reclassify the building as partially open; thereby increasing wind coefficients. Increased wind coefficients may require additional girts and purlins, as well as increased wind reactions of the rigid frames. If you are using overhead doors, or any other material to enclose your building which has not been designed for full wind load, notify Bigbee Steel Buildings, Inc. in advance so that your building can be properly designed for partially open conditions.

BUILDING APPEARANCE

6.1 All metal panels & flashings exhibit some oil-canning or wave of the surface in certain conditions. Panels & flashings that are fixed to insulated structurals so that differential movement between the two occur, may exhibit more pronounced oil-canning when subjected to rising temperatures. Oil-Canning is a natural occurrence in metal panels & flashings and that does **NOT** affect the finish or structural integrity of the panel & flashing and is therefore **NOT** a cause for rejection.

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CUSTOM STEEL BUILDINGS
P.O. BOX 2314
MUSCLE SHOALS, ALABAMA 35662
PHONE:(256) 383-7322 FAX:(256) 381-9869

NOTE TO ERECTOR: Roof Curbs, Roof Jacks/Boots, Roof Vents, Doors, Windows, Fans, Shutters, Louvers, etc.), when supplied by Bigbee Steel Buildings, Inc., **MUST** be installed per the Component Manufacturer's supplied installation instructions.

THE ERECTION INFORMATION PRESENTED ON THIS DETAIL SHEET IS PROVIDED AS A BUILDING SUPPLEMENT TO THE ERECTIONS DRAWINGS SUPPLIED WITH YOUR BIGBEE METAL BUILDING.

ILLUSTRATIONS AND TEXT ARE TYPICAL FOR MOST STANDARD BIGBEE BUILDINGS, ALTHOUGH VARIATIONS MAY OCCUR DUE TO SPECIFIC CUSTOMER REQUIREMENTS OR SUBSEQUENT ENGINEERING CHANGES.

FOR THESE REASONS, ALWAYS REFER TO THE ERECTION DRAWINGS SUPPLIED WITH THE BUILDING SHIPMENT BEFORE BEGINNING THE ERECTION PROCESS. IF PROPER DETAILS ARE NOT SHOWN OR A QUESTION ARISES, IT IS RECOMMENDED THAT YOU CONTACT BIGBEE STEEL BUILDINGS, INC. FOR FURTHER INFORMATION.
(WATTS: 1-800-633-3378) (WEB: www.bigbee.com)

SAFETY

BIGBEE STEEL BUILDINGS, INC IS COMMITTED TO THE MANUFACTURE OF QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF BIGBEE STEEL BUILDINGS, INC.

BIGBEE STEEL BUILDINGS, INC. INTENDS THAT THIS DETAIL SHEET BE INTERPRETED AND ADMINISTERED WITH SOUND JUDGMENT CONSISTENT WITH GOOD SAFETY PRACTICES.

REVISIONS

NO	DATE	BY	DESCRIPTION
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DESIGN CERTIFICATION

CERTIFICATION OF THE METAL BUILDING COMPONENTS SUPPLIED BY BIGBEE STEEL BUILDINGS, INC. IS LIMITED TO THE LOADS SPECIFIED ON THE BIGBEE STEEL BUILDINGS, INC. PURCHASE ORDER. THE BUILDING MUST BE ERECTED IN ACCORDANCE WITH THE ERECTION DRAWINGS AND EXCLUDES ITEMS NOT SUPPLIED BY BIGBEE STEEL BUILDINGS, INC. THE UNDERSIGNED IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT.

IAS ACCREDITED Metal Building Systems AC 472

GEORGIA REGISTERED No. PE016914 PROFESSIONAL ENGINEER EVANS FARNUM

MEMBER MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

DRAWING NAME: GENERAL NOTES

CUSTOMER: PINELAND CONTRACTORS METTER, GA

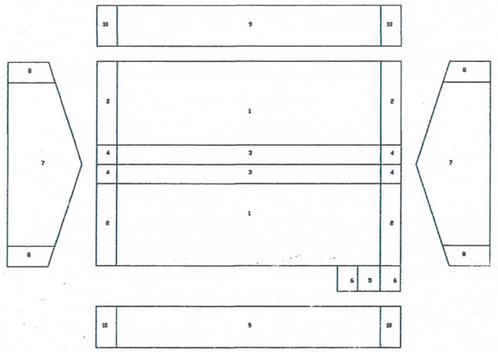
PROJECT: EVANS CO PUBLIC WORKS CLAXTON, GA

FILENAME: 16854-1 - DS-Dwgs

JOB #: 16854

DRAWN | DESIGN | CHECKED | SCALE

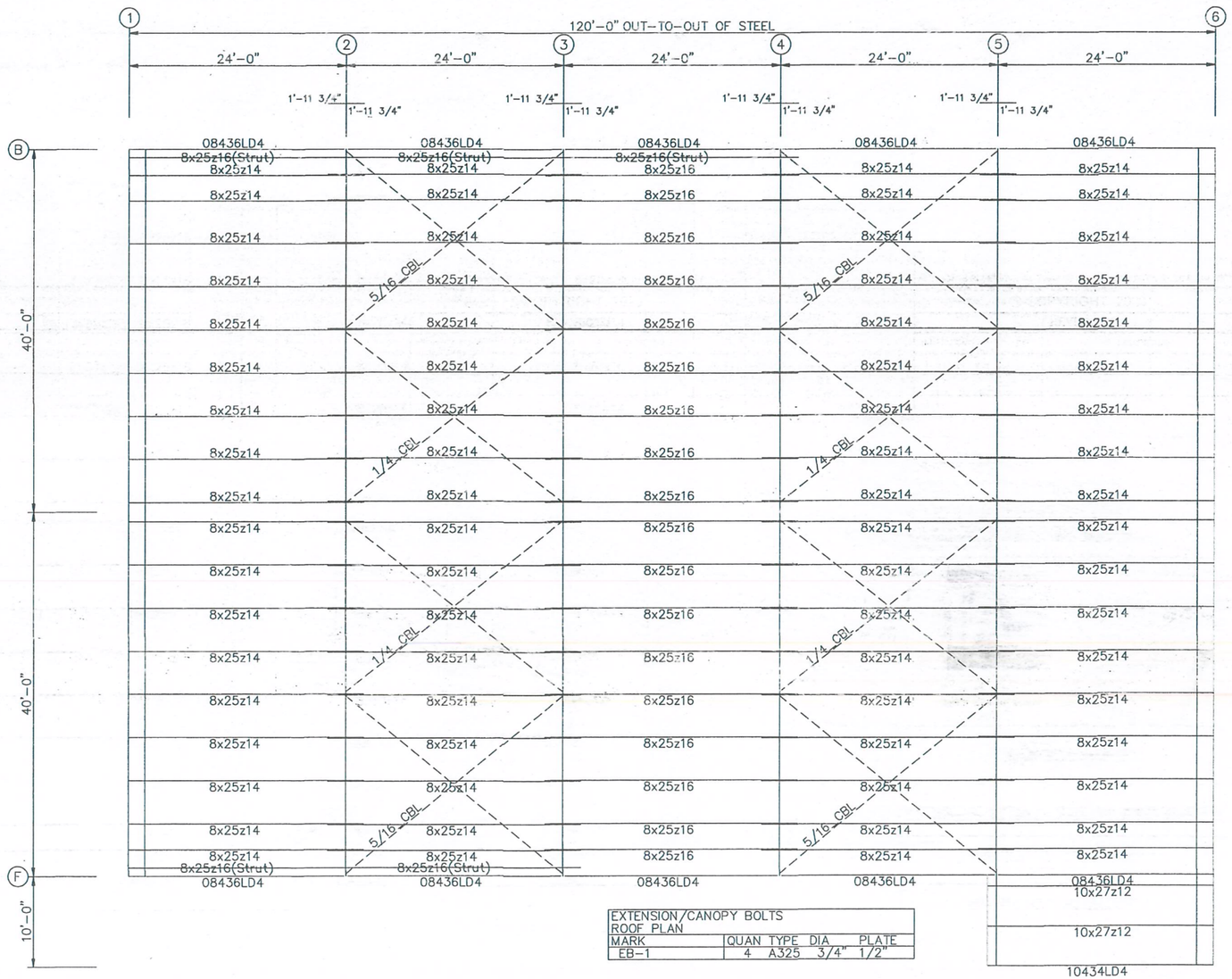
DRAWING #: D-0



Components & Cladding

Zone	Width (ft)	Length (ft)	Pressure(psf) Member	Pressure(psf) Panel	Suction(psf) Member	Suction(psf) Panel
1			16.00	16.00	-16.00	-43.60
2		8.00	16.00	16.00	-28.80	-63.66
3	8.00		16.00	16.00	-28.80	-63.66
4	8.00	8.00	16.00	16.00	-43.20	-75.43
5	10.54	9.00	16.00	16.00	-30.00	-50.00
6	10.54	8.00	16.00	16.00	-33.60	-82.00
7			19.00	23.60	-21.01	-25.60
8	8.00		19.00	23.60	-22.48	-31.49
9			19.00	23.60	-21.00	-25.60
10	8.00		19.00	23.60	-22.47	-31.49

(+) wind towards surface
(-) wind away from surface



EXTENSION/CANOPY BOLTS ROOF PLAN				
MARK	QUAN	TYPE	DIA	PLATE
EB-1	4	A325	3/4"	1/2"

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MUSCLE SHOALS, ALABAMA 35662
PHONE:(256) 383-7322 FAX:(256) 381-9669

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REVISIONS

NO	DATE	BY	DESCRIPTION
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2			
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DESIGN CERTIFICATION

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IAS ACCREDITED Metal Building Systems AC 472

GEORGIA REGISTERED PROFESSIONAL ENGINEER WILLIAM S. FARMER

MEMBER MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

DRAWING NAME: DESIGN SUMMARY

CUSTOMER: PINELAND CONTRACTORS METTER, GA

PROJECT: EVANS CO PUBLIC WORKS CLAXTON, GA

FILENAME: 16854-1 - DS-Dwgs

JOB #: 16854

DRAWN | DESIGN | DRAWING #: DS-1

CHECKED | SCALE: NONE

APPROVED | DATE

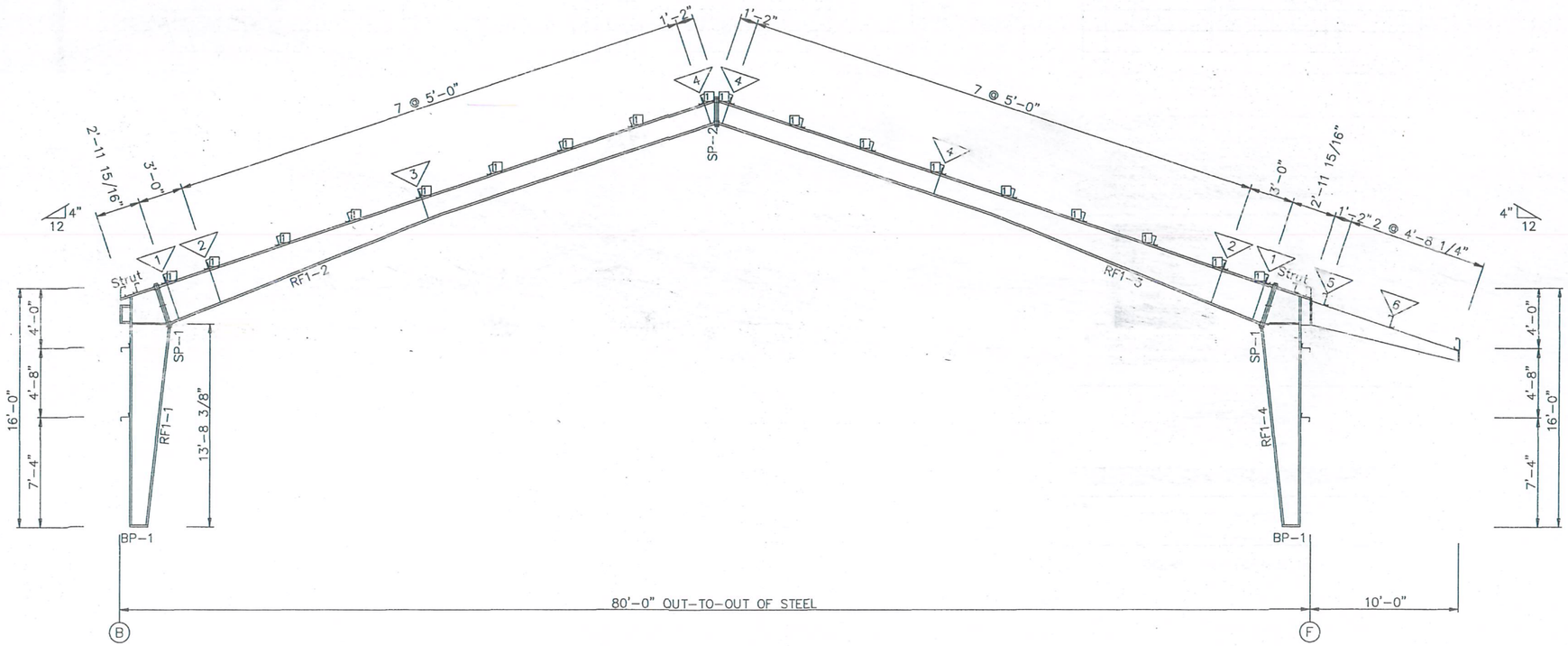
SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	2	A325	3/4"	2 1/4"	8"	1/2"	3'-1 1/4"
SP-2	4	4	0	A325	3/4"	2 1/4"	6"	1/2"	2'-0"

FLANGE BRACE TABLE FRAME LINE 2 3 4 5				
▽ID	# SIDES	MARK	LENGTH	OFFSET
1	1	FB37.5L	3'-1 1/2"	1'-10 3/4"
2	1	FB35.8L	2'-11 3/4"	1'-10 3/4"
3	1	FB28L	2'-4"	1'-10 3/4"
4	1	FB27.8L	2'-3 3/4"	1'-10 3/4"
5	1	FB-L		1'-10 3/4"
6	1	FB-L		1'-10 3/4"

BASE PLATE TABLE			
Col Mark	Plate Size Width	Thick	Length
BP-1	8"	1/2"	1'-0 1/4"

MEMBER TABLE					
Mark	Web Depth Start/End	Web Plate Thick	Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF1-1	11.5/29.5	0.188	15'-11 7/8"	8 x 1/4" x 15'-5 1/2" 8 x 1/4" x 2'-4 1/16"	8 x 5/16" x 13'-5 1/8"
RF1-2	29.5/15.5 15.5/15.5	0.135 0.135	19'-8 13/16" 19'-11"	8 x 1/4" x 20'-0 13/16" 8	8 x 5/16" x 19'-9 3/16" 8 x 1/4" x 19'-5 3/4"
RF1-3	15.5/15.5 15.5/29.5	0.135 0.135	19'-11" 19'-8 13/16"	8 x 1/4" x 19'-7" 8 x 1/4" x 20'-0 13/16"	8 x 1/4" x 19'-5 3/4" 8 x 5/16" x 19'-9 3/16"
RF1-4	29.5/11.5	0.188	15'-11 7/8"	8 x 1/4" x 2'-3 7/8" 8 x 1/4" x 15'-5 1/2"	8 x 5/16" x 13'-5 1/8"
EB-1	19/7.5	0.135	10'-9 1/8"	6 x 1/4" x 10'-9 1/8"	6 x 1/4" x 10'-9 1/8"

CONNECTION PLATES	
☐ID	Mark/Part
1	AR-8G



RIGID FRAME ELEVATION: FRAME LINE 2 3 4 5

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REVISIONS

NO	DATE	BY	DESCRIPTION
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DESIGN CERTIFICATION

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IAS ACCREDITED Metal Building Systems AC 472

GEORGIA REGISTERED PROFESSIONAL ENGINEER WILLIAM S. FARNEMAN

MEMBER MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

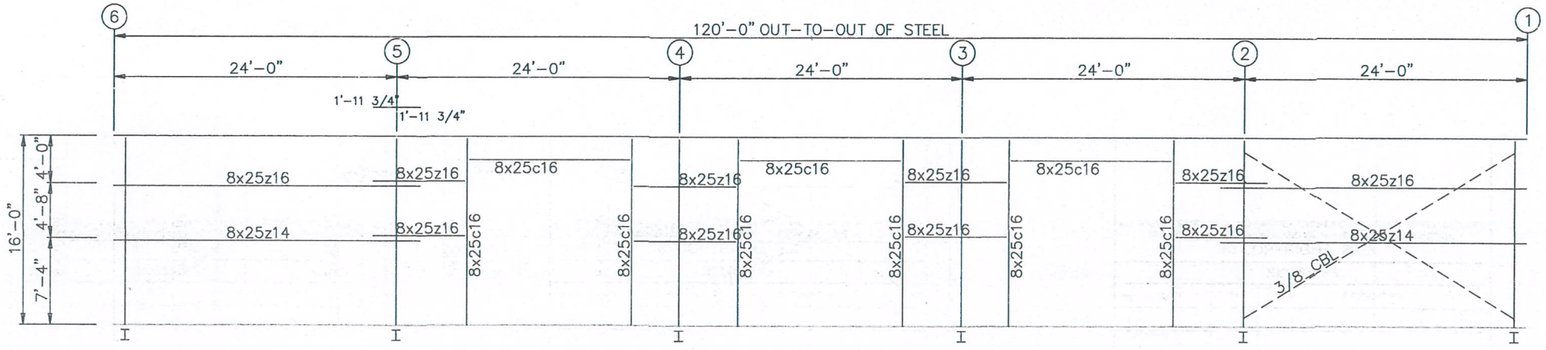
DRAWING NAME:
DESIGN SUMMARY

CUSTOMER:
PINELAND CONTRACTORS
METTER, GA

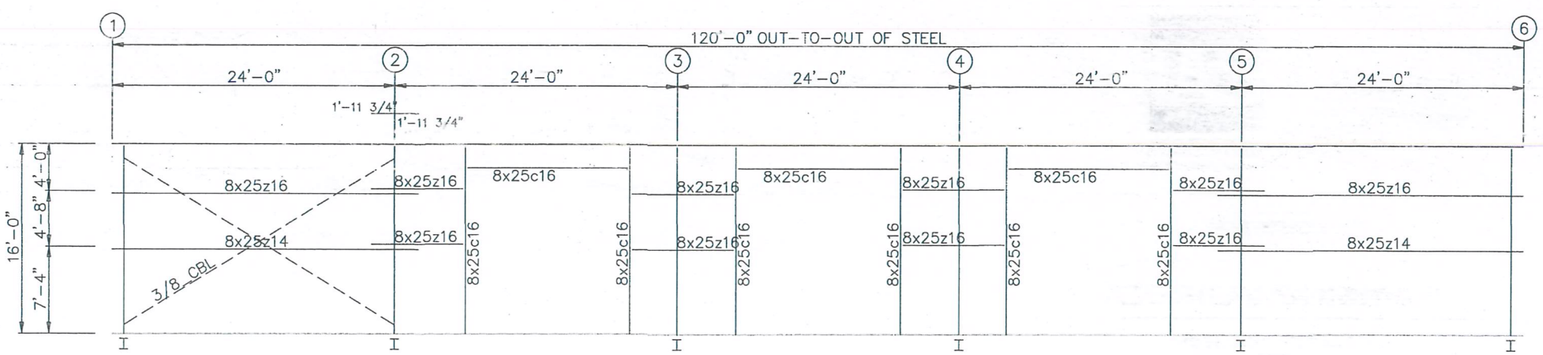
PROJECT:
EVANS CO PUBLIC WORKS
CLAXTON, GA

FILENAME:	JOB #
16854-1 - DS-Dwgs	16854
DRAWN	DESIGN
CHECKED	SCALE NONE

DRAWING #
DS-2



SIDEWALL FRAMING: FRAME LINE B



SIDEWALL FRAMING: FRAME LINE F

BIGBEE
CUSTOM STEEL BUILDINGS
P.O. BOX 2314
MUSCLE SHOALS, ALABAMA 35662
PHONE:(256) 383-7322 FAX:(256) 381-9669

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IAS ACCREDITED Metal Building Systems AC 472

GEORGIA REGISTERED PROFESSIONAL ENGINEER WILLIAM S. FARNELAND

MEMBER MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

DRAWING NAME:
DESIGN SUMMARY

CUSTOMER:
PINELAND CONTRACTORS
METTER, GA

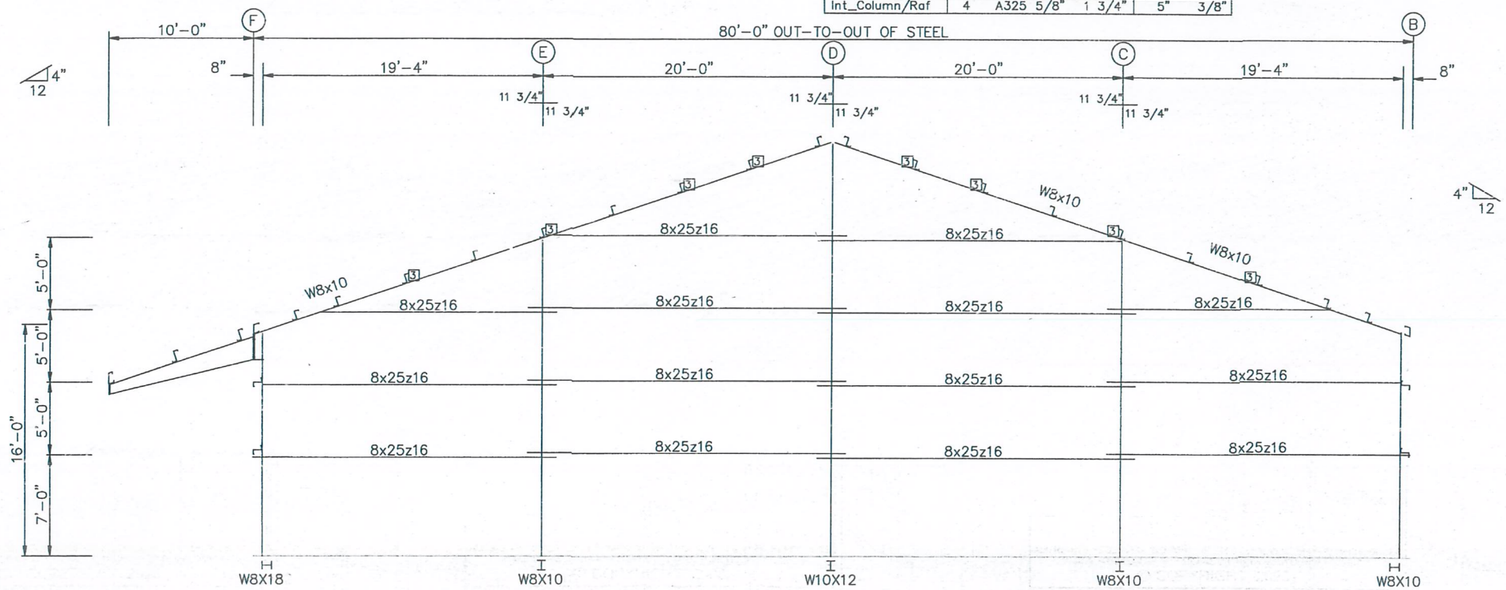
PROJECT:
EVANS CO PUBLIC WORKS
CLAXTON, GA

FILENAME:	JOB #
16854-1 - DS-Dwgs	16854
DRAWN	DESIGN
CHECKED	SCALE NONE

DRAWING # DS-3

BOLT & PLATE TABLE: FRAME LINE 6						
LOCATION	QTY	TYPE	DIA	LENGTH	WIDTH	THICK
EC-1/ER-1	4	A325	5/8"	1 3/4"	5"	3/8"
ER-2/ER-3	4	A325	5/8"	1 3/4"	5"	3/8"
ER-1/ER-3	4	A325	5/8"	2 1/4"	5"	1/2"
EC-6/ER-2	4	A325	5/8"	1 3/4"	6"	3/8"
Int_Column/Raf	4	A325	5/8"	1 3/4"	5"	3/8"
Int_Column/Raf	4	A325	5/8"	1 3/4"	5"	3/8"

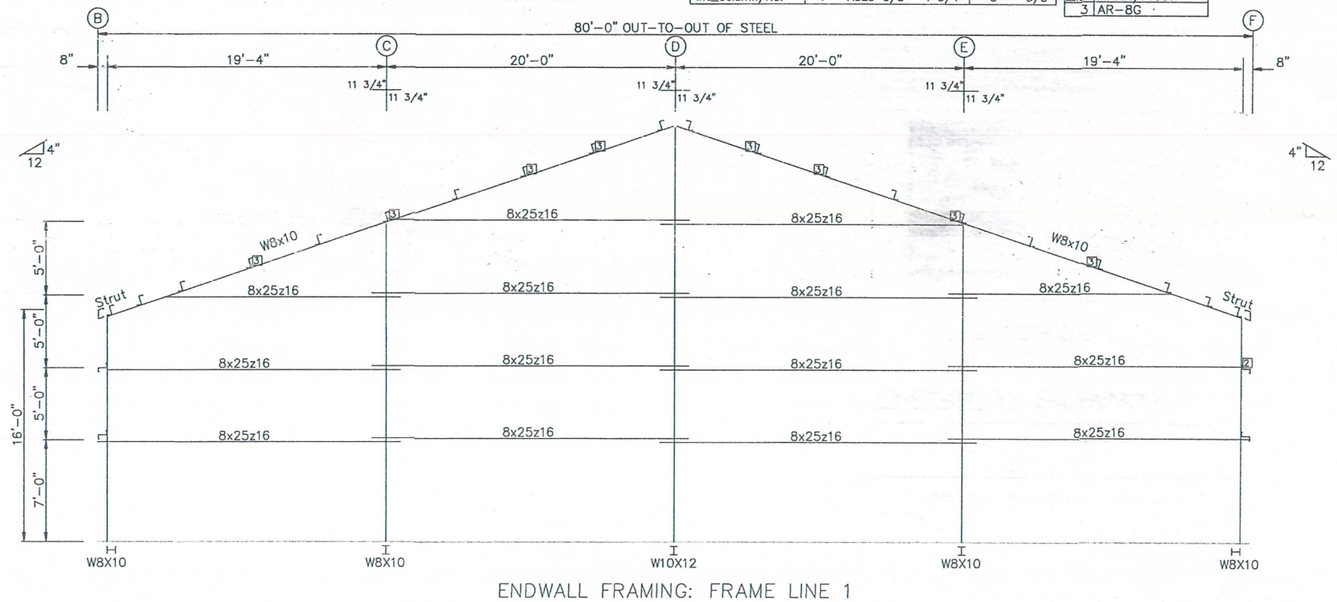
CONNECTION PLATES FRAME LINE 6	
ID	MARK/PART
3	AR-8G



ENDWALL FRAMING: FRAME LINE 6

BOLT & PLATE TABLE: FRAME LINE 1						
LOCATION	QTY	TYPE	DIA	LENGTH	WIDTH	THICK
Cor_Column/Raf	4	A325	5/8"	1 3/4"	5"	3/8"
FR-1/ER-1	4	A325	5/8"	2 1/4"	5"	1/2"
Int_Column/Raf	4	A325	5/8"	1 3/4"	5"	3/8"
Int_Column/Raf	4	A325	5/8"	1 3/4"	5"	3/8"

CONNECTION PLATES FRAME LINE 1	
ID	MARK/PART
3	AR-8G



ENDWALL FRAMING: FRAME LINE 1

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CUSTOM STEEL BUILDINGS
P.O. BOX 2314
MUSCLE SHOALS, ALABAMA 35662
PHONE:(256) 383-7322 FAX:(256) 381-9669

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REVISIONS			
NO	DATE	BY	DESCRIPTION
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IAS ACCREDITED Metal Building Systems AC 472

GEORGIA No. PE011814 PROFESSIONAL ENGINEER WILLIAM S. FARNELL

M E M B E R MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

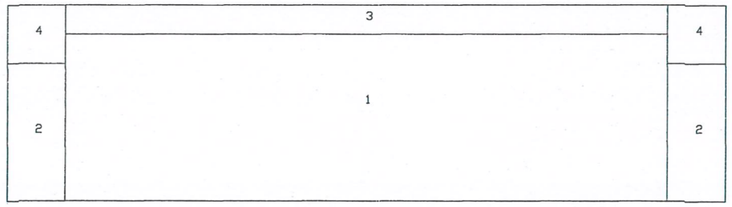
DRAWING NAME:
DESIGN SUMMARY

CUSTOMER:
PINELAND CONTRACTORS
METTER, GA

PROJECT:
EVANS CO PUBLIC WORKS
CLAXTON, GA

FILENAME:	JOB #
16854-1 - DS-Dwgs	16854
DRAWN	DESIGN
CHECKED	SCALE NONE

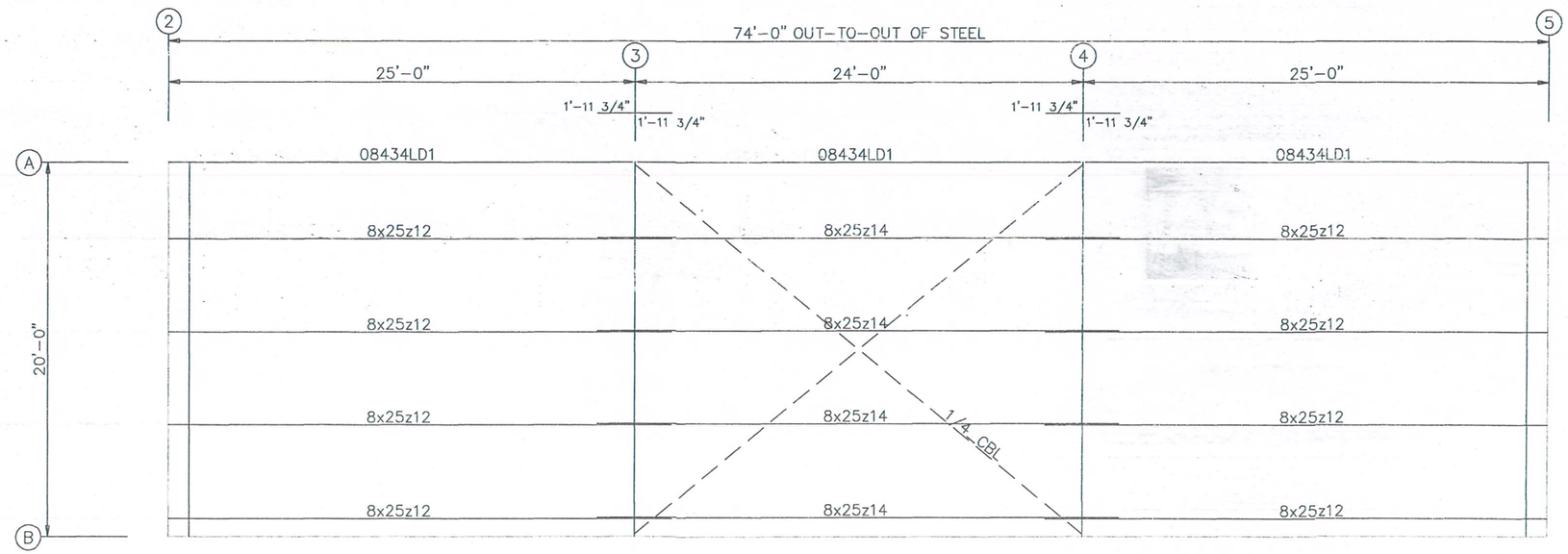
DRAWING # DS-4



Components & Cladding

Zone	Width (ft)	Length (ft)	Pressure(psf) Member	Pressure(psf) Panel	Suction(psf) Member	Suction(psf) Panel
1			16.00	16.00	-22.38	-22.38
2		6.00	16.00	16.00	-29.32	-31.11
3	3.00		16.00	16.00	-24.17	-25.96
4	6.00	6.00	16.00	16.00	-24.17	-34.69

(+) wind towards surface
(-) wind away from surface



ROOF FRAMING PLAN

BIGBEE
CUSTOM STEEL BUILDINGS
P.O. BOX 2314
MUSCLE SHOALS, ALABAMA 35662
PHONE:(256) 383-7322 FAX:(256) 381-9669

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REVISIONS

NO	DATE	BY	DESCRIPTION
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DESIGN CERTIFICATION

CERTIFICATION OF THE METAL BUILDING COMPONENTS SUPPLIED BY BIGBEE STEEL BUILDINGS, INC. IS LIMITED TO THE LOADS SPECIFIED ON THE BIGBEE STEEL BUILDINGS, INC. PURCHASE ORDER. THE BUILDING MUST BE ERECTED IN ACCORDANCE WITH THE ERECTION DRAWINGS AND EXCLUDES ITEMS NOT SUPPLIED BY BIGBEE STEEL BUILDINGS, INC. THE UNDERSIGNED IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT.

IAS ACCREDITED Metal Building Systems AC 472

GEORGIA REGISTERED No. PE013911 PROFESSIONAL ENGINEER WILLIAM S. FARNHAM

MEMBER MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

DRAWING NAME:
DESIGN SUMMARY

CUSTOMER:
PINELAND CONTRACTORS
METTER, GA

PROJECT:
EVANS CO PUBLIC WORKS
CLAXTON, GA

FILENAME:	JOB #
16854-1 - DS-Dwgs	16854

DRAWN	DESIGN	DRAWING #
CHECKED	SCALE NONE	DS-5